



# Basic Modeling & Analysis By Example

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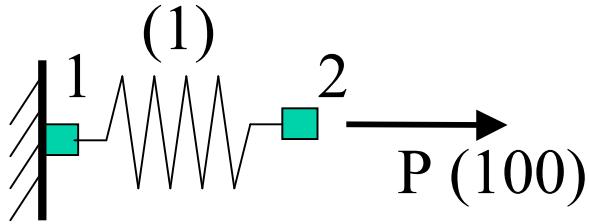
OpenSees Days 2012

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# Spring Example - Load Control

a.tcl



```
# create the model builder
model Basic -ndm 1 -ndf 1

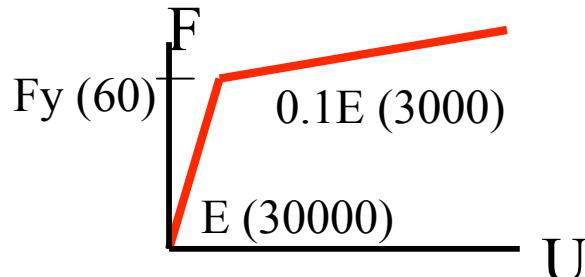
# create 2 nodes
node 1 0.0
node 2 0.0

# fix node 1
fix 1 1

# create material
set Fy 60.0
set E 30000.0
set b 0.1
uniaxialMaterial Steel01 1 $Fy $E $b

# create element
element zeroLength 1 1 2 -mat 1 -dir 1

# create time series and load pattern
set P 100.0
timeSeries Linear 1
pattern Plain 1 1 {
    load 2 $P
}
```



```
# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-6 6 0
algorithm Newton
system ProfileSPD
integrator LoadControl 0.1
analysis Static

# perform the analysis
analyze 10

# Output
print node 2

set exact [expr $Fy/$E + (100-$Fy)/($b*$E)]
set res [lindex [nodeDisp 2] 0]
if {$exact == $res} {
    puts "Exact ($exact) EQUALS Result ($res)"
} else {
    puts "Exact ($exact) NOT EQUAL Result ($res)"
}
```

```
Terminal — bash — 80x19
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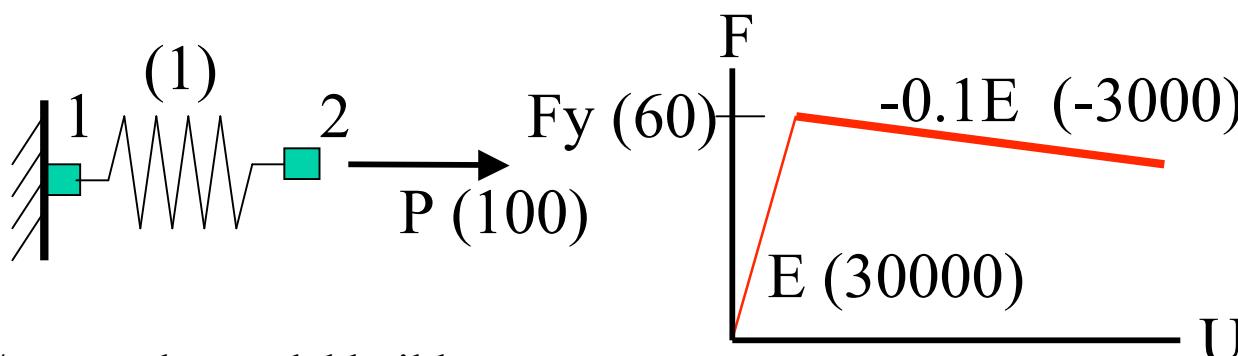
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Node: 2
    Coordinates : 0
    Disps: 0.0153333
    Velocities : 0
    unbalanced Load: 100
    ID : 0

Exact (0.0153333333333334) IS NOT EQUAL TO Result (0.01533333333333250)
fmk:~/Desktop/Workshops/ChinaWorkshop2011/examn
```



Computers cannot represent all numbers exactly  
and  
Computer math involves roundoff



```
# create the model builder
model Basic -ndm 1 -ndf 1
# create 2 nodes
node 1 0.0
node 2 0.0
# fix node 1
fix 1 1
# create material
set Fy 60.0
set E 30000.0
set b -0.1
uniaxialMaterial Steel01 1 $Fy $E $b
# create element
element zeroLength 1 1 2 -mat 1 -dir 1
# create time series and load pattern
set P 100.0
timeSeries Linear 1
pattern Plain 1 1 {
    load 2 $P
}
```

```
# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-6 6 0
algorithm Newton
system ProfileSPD
integrator LoadControl 0.1
analysis Static

# perform the analysis
analyze 10
# Output
print node 2
```

b.tcl

```
Terminal — bash — 87x20

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ProfileSPDLinDirectSolver::solve() - aii < 0 (i, aii): (0,0)
WARNING NewtonRaphson::solveCurrentStep() -the LinearSysOfEqn failed in solve()
StaticAnalysis::analyze() - the Algorithm failed at iteration: 5 with domain at load fa
ctor 0.6
OpenSees > analyze failed, returned: -3 error flag

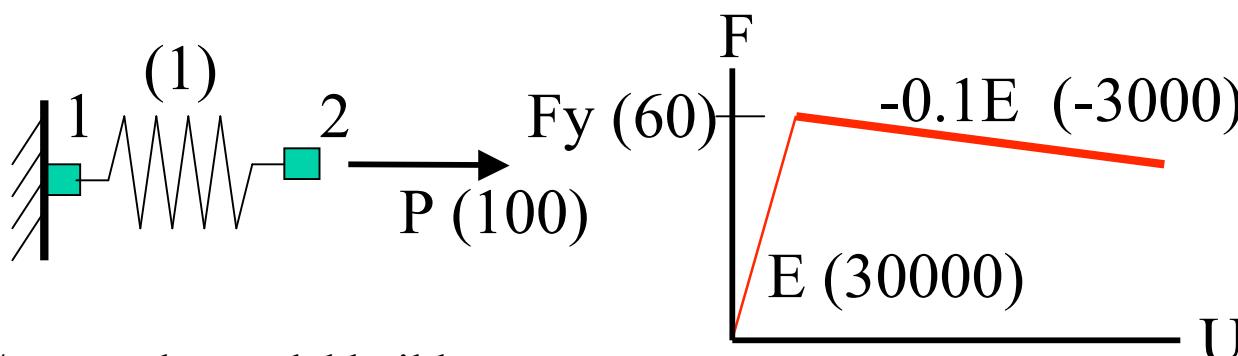
Node. 2
    Coordinates : 0
    Disps: 0.00166667
    Velocities : 0
    unbalanced Load: 50
    ID : 0

fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$
```

!

change **system ProfileSPD** to **system BandGen**

change **LoadControl 0.1** to **LoadControl 0.0999999**



```

# create the model builder
model Basic -ndm 1 -ndf 1
# create 2 nodes
node 1 0.0
node 2 0.0
# fix node 1
fix 1 1

# create material
set Fy 60.0
set E 30000.0
set b -0.1
uniaxialMaterial Steel01 1 $Fy $E $b

# create element
element zeroLength 1 1 2 -mat 1 -dir 1

# create time series and load pattern
set P 100.0
timeSeries Linear 1
pattern Plain 1 1 {
    load 2 $P
}

```

```

# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-6 6 0
algorithm Newton
system BandGen
integrator LoadControl 0.09999
analysis Static

```

```

# perform the analysis
analyze 10
# Output
print node 2

```

c.tcl

```

Terminal — bash — 104x22
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WARNING: CTestNormDispIncr::test() - failed to converge
after: 6 iterations
NewtnRaphson::solveCurrentStep() -the ConvergenceTest object failed in test()
StaticAnalysis::analyze() - the Algorithm failed at iteration: 6 with domain at load factor 0.7
OpenSees > analyze failed, returned: -3 error flag

Node: 2
  Coordinates : 0
  Disps: 0.002
  Velocities : 0
    unbalanced Load: 60
  ID : 0

fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ 

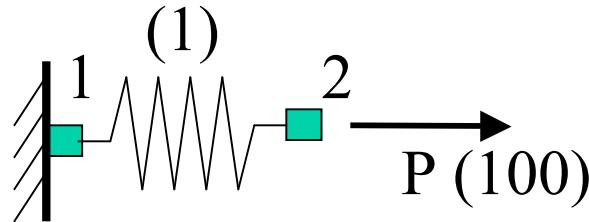
```

With a Yield Strength of 60  
 This is as far as we can push the  
 Model using LoadControl

**We can go further using a Displacement Control scheme**

# Spring Example - Displacement Control

d.tcl

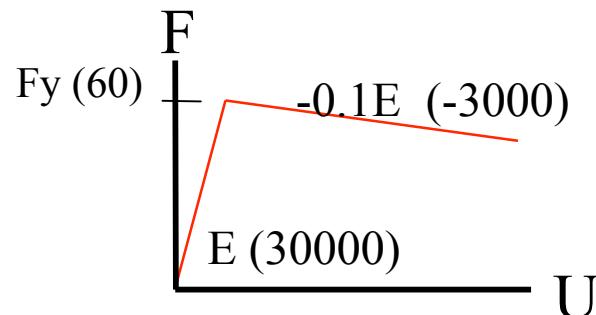


```
# create the model builder
model Basic -ndm 1 -ndf 1
# create 2 nodes
node 1 0.0
node 2 0.0
# fix node 1
fix 1 1

# create material
set Fy 60.0
set E 30000.0
set b -0.1
uniaxialMaterial Steel01 1 $Fy $E $b

# create element
element zeroLength 1 1 2 -mat 1 -dir 1

# create time series and load pattern
set P 100.0
timeSeries Linear 1
pattern Plain 1 1 {
    load 2 $P
}
```



```
# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-12 6 0
algorithm Newton
system BandGen
integrator DisplacementControl 2 1 0.001
analysis Static

# perform the analysis & print results
for {set i 0} {$i < 10} {incr i 1} {
    analyze 1
    set factor [getTime]
    puts "[expr $factor*$P] [lindex [nodeDisp 2] 0]"
}

print node 2
```



Terminal — bash — 92x30

fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples\$ OpenSees d.tcl

```
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```

```
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```

```
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```

```
30.0 0.00100000000000000002  
60.0 0.00200000000000000004  
56.99999999999999 0.00300000000000000006  
54.0 0.00400000000000000008  
51.0 0.00500000000000000010  
48.0 0.00600000000000000012  
45.0 0.00700000000000000015  
42.0 0.00800000000000000017  
39.0 0.009000000000000000105  
36.0 0.010000000000000000194
```

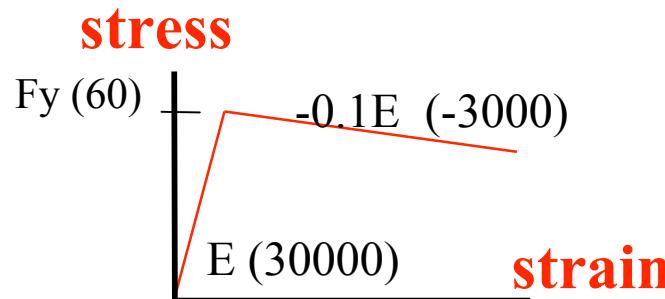
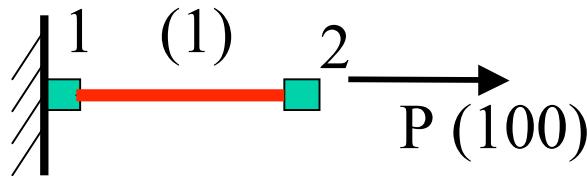
```
Node: 2
```

```
    Coordinates : 0  
    Disps: 0.01  
    Velocities : 0  
        unbalanced Load: 36  
    ID : 0
```

fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples\$

# Truss Example - Displacement Control

e.tcl



```
# create the model builder
model Basic -ndm 1 -ndf 1

# create 2 nodes
node 1 0.0
node 2 1.0
# fix node 1
fix 1 1

# create material
set Fy 60.0
set E 30000.0
set b -0.1
uniaxialMaterial Steel01 1 $Fy $E $b

# create element
set A 1.0
element Truss 1 1 2 $A 1

# create time series and load pattern
set P 100.0
timeSeries Linear 1
pattern Plain 1 1 {
    load 2 $P
}

# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-12 6 0
algorithm Newton
system BandGen
integrator DisplacementControl 2 1 0.001
analysis Static

# perform the analysis & print results
for {set i 0} {$i < 10} {incr i 1} {
    analyze 1
    set factor [getTime]
    puts "[expr $factor*$P] [lindex [nodeDisp 2] 0]"
}

print node 2
```

```
Terminal — bash — 92x30
fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ OpenSees e.tcl

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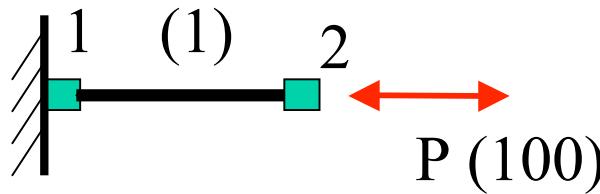
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30.0  0.00100000000000000002
60.0  0.00200000000000000004
56.99999999999999  0.00300000000000000006
54.0  0.00400000000000000008
51.0  0.00500000000000000010
48.0  0.00600000000000000012
45.0  0.00700000000000000015
42.0  0.00800000000000000017
39.0  0.009000000000000000105
36.0  0.010000000000000000194

Node: 2
    Coordinates : 1
    Disps: 0.01
    Velocities : 0
    unbalanced Load: 36
    ID : 0

fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ 
```

# Truss Example - Push & Pull



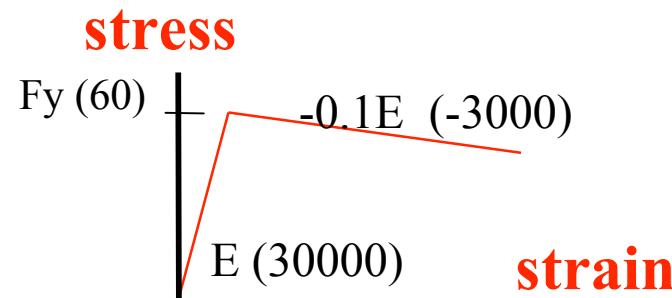
```
# create the model builder
model Basic -ndm 1 -ndf 1

# create 2 nodes
node 1 0.0
node 2 1.0
# fix node 1
fix 1 1

# create material
set Fy 60.0
set E 30000.0
set b -0.1
uniaxialMaterial Steel01 1 $Fy $E $b

# create element
set A 1.0
element Truss 1 1 2 $A 1

# create time series and load pattern
set P 100.0
timeSeries Linear 1
pattern Plain 1 1 {
    load 2 $P
}
```



```
# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-12 6 0
algorithm Newton
system BandGen
integrator DisplacementControl 2 1 0.001
analysis Static

# perform the analysis & print results
foreach {$numIter $dU} {10 0.001 20 -0.001 10 0.001} {
    integrator DisplacementControl 2 1 0.001
    analyze $numIter
    set factor [getTime]
    puts "[expr $factor*$P] [lindex [nodeDisp 2] 0]"
}

print node 2
```

```
Terminal — bash — 93x23
fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ OpenSees f.tcl

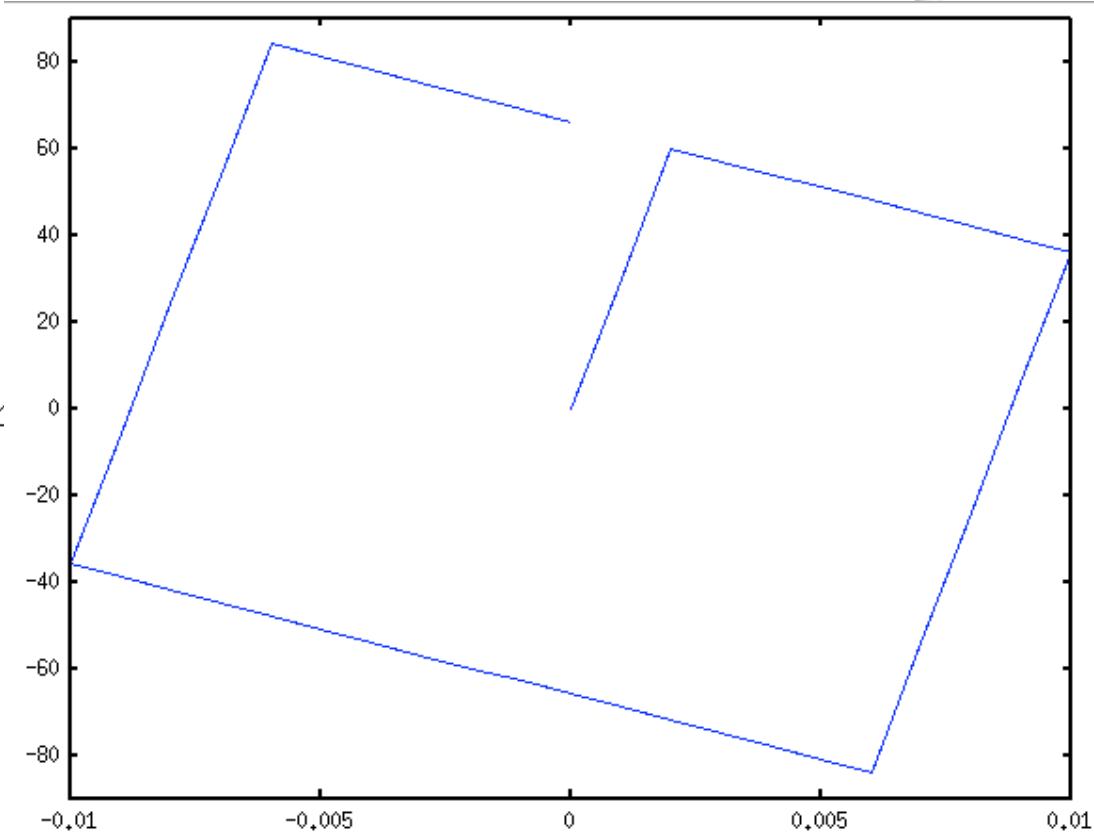
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```

```
36.0 0.0100000000000000194
-36.0 -0.0100000000000000194
66.0 0.00000000000000000000
```

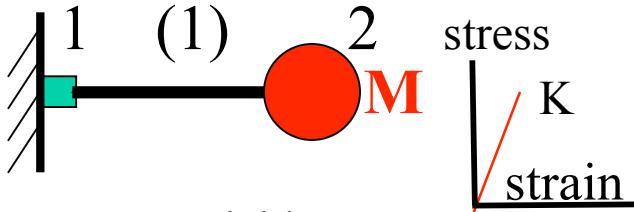
```
Node: 2
    Coordinates : 1
    Disps: 0
    Velocities : 0
        unbalanced Load: 66
    ID : 0
```

```
fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$
```



# Truss Example - Uniform Excitation

j.tcl



```
# set some variables
set Tn 1.0
set K 4.0
set dampR 0.02
#set some constants
set g 386.4
set PI [expr 2.0 * asin(1.0)]
#derived quantities
set Wn [expr 2.0 * $PI / $Tn]
set M [expr $K / ($Wn * $Wn)]
set c [expr 2.0*$M*$Wn*$dampR]

# create the model
model basic -ndm 1 -ndf 1
node 1 0.0
node 2 1.0 -mass $M
fix 1 1
uniaxialMaterial Elastic 1 $K 0.0
uniaxialMaterial Elastic 2 0.0 $c
uniaxialMaterial Parallel 3 1 2

element truss 1 1 2 1.0 3
set dT 0.0;
set nPts 0;
```

```
# create the uniform excitation pattern
set record el centro
source ReadRecord.tcl
ReadRecord $record.AT2 $record.dat dT nPts
timeSeries Path 1 -filePath $record.dat -dt $dT -factor $;
pattern UniformExcitation 1 1 -accel 1

# create the analysis
constraints Plain
integrator Newmark 0.5 [expr 1.0/6.0]
system ProfileSPD
test NormUnbalance 1.0e-12 6 0
algorithm Newton
numberer RCM
analysis Transient

# perform analysis
set t 0.0; set ok 0.0; set maxD 0.0;
set maxT [expr (1+$nPts)*$dT];
while {$ok == 0 && $t < $maxT} {
    set ok [analyze 1 $dT]
    set t [getTime]
    set d [nodeDisp 2 1]
    if {$d > $maxD} {
        set maxD $d
    } elseif {$d < [expr -$maxD]} {
        set maxD [expr -$d]
    }
}
puts "record: $record period: $Tn damping ratio: $dampR r
```

```
Terminal — bash — 81x13
examples> OpenSees j.tcl

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record: el_centro period: 1.0 damping ratio: 0.02 max disp: 5.962305018001343
examples> █
```

# Elastic Portal Frame - Pushover

g.tcl

```
# create the model builder
model Basic -ndm 2 -ndf 3

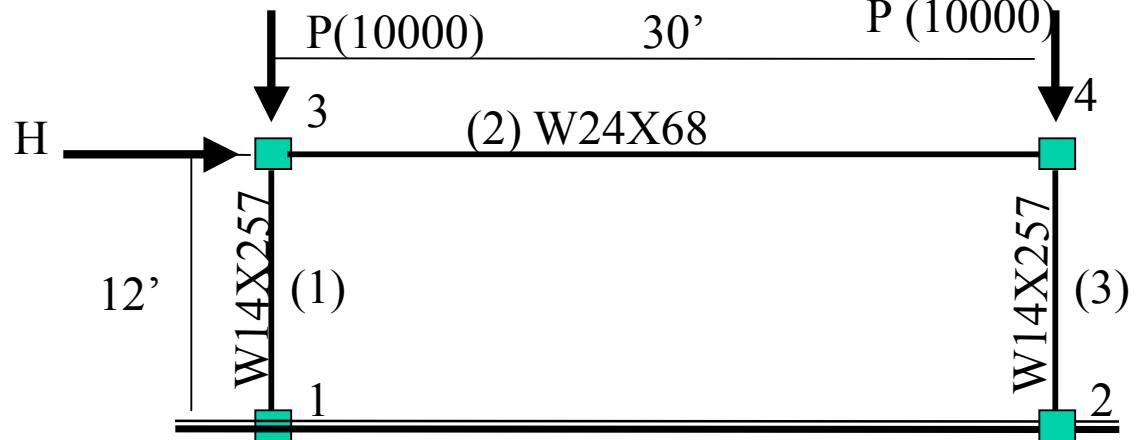
# create 2 nodes
node 1 0.0 0.0
node 2 360.0 0.0
node 3 0.0 144.0
node 4 360.0 144.0

# fix node 1
fix 1 1 1 1
fix 2 1 1 1

geomTransf Linear 1
geomTransf Linear 2

# create elements
source SteelWSections.tcl
set in 1.0
set E 30000
ElasticBeamWSection2d 1 1 3 W14X257 $E 1
ElasticBeamWSection2d 2 3 4 W24X68 $E 2
ElasticBeamWSection2d 3 2 4 W14X257 $E 1

# create time series and load pattern
set P 10000.0
timeSeries Constant 1
pattern Plain 1 1 {
    load 3 0.0 -$P 0.0
    load 4 0.0 -$P 0.0
}
```



```
# create an analysis
constraints Plain
numberer RCM
test NormDispIncr 1.0e-12 6 0
algorithm Newton
system ProfileSPD
integrator LoadControl 1.0
analysis Static

# perform the analysis
analyze 1
timeSeries Linear 2
pattern Plain 2 2 {
    load 3 1.0 0.0 0.0
    load 4 1.0 0.0 0.0
}
integrator DisplacementControl 3 1 0.1
analyze 100; print node 3
```

# SteelWSections.tcl

```
proc ElasticBeamWSection2d {eleTag iNode jNode sectType E transfTag {Orient XX}} {
    global WSection
    global in
    set found 0
    foreach {section prop} [array get WSection $sectType] {
        set propList [split $prop]
        set A [expr [lindex $propList 0]*$in*$in]
        set Ixx [expr [lindex $propList 5]*$in*$in*$in*$in]
        set Iyy [expr [lindex $propList 6]*$in*$in*$in*$in]
        if {$Orient == "YY" } {
            puts "element elasticBeamColumn $eleTag $iNode $jNode $A $E $Iyy $transfTag"
            element elasticBeamColumn $eleTag $iNode $jNode $A $E $Iyy $transfTag
        } else {
            puts "element elasticBeamColumn $eleTag $iNode $jNode $A $E $Ixx $transfTag"
            element elasticBeamColumn $eleTag $iNode $jNode $A $E $Ixx $transfTag
        }
    }
}
#Winxlb/f "Area(in2) d(in) bf(in) tw(in) tf(in) Ixx(in4) Iyy(in4)"
array set WSection {
    W44X335    "98.5 44.0 15.9 1.03 1.77 31100 1200 74.7"
    W44X290    "85.4 43.6 15.8 0.865 1.58 27000 1040 50.9"
    W44X262    "76.9 43.3 15.8 0.785 1.42 24100 923 37.3"
    W44X230    "67.7 42.9 15.8 0.710 1.22 20800 796 24.9"
    W40X593    "174 43.0 16.7 1.79 3.23 50400 2520 445"
    W40X503    "148 42.1 16.4 1.54 2.76 41600 2040 277"
}
```

Terminal — bash — 101x22

```
fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ OpenSees g.tcl
```

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Pacific Earthquake Engineering Research Center -- 2.3.0.alpha

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```
element elasticBeamColumn 1 1 3 75.6 30000 3400.0 1
element elasticBeamColumn 2 3 4 20.1 30000 1830.0 2
element elasticBeamColumn 3 2 4 75.6 30000 3400.0 1

Node: 3
    Coordinates : 0 144
    Disps: 10 -0.609542 -0.0787731
    unbalanced Load: 1765.38 -10000 0
    ID : 3 4 5
```

```
fmk:~/Desktop/Workshops/ChinaWorkshop2011/example
```

# Elastic Portal Frame - Pushover

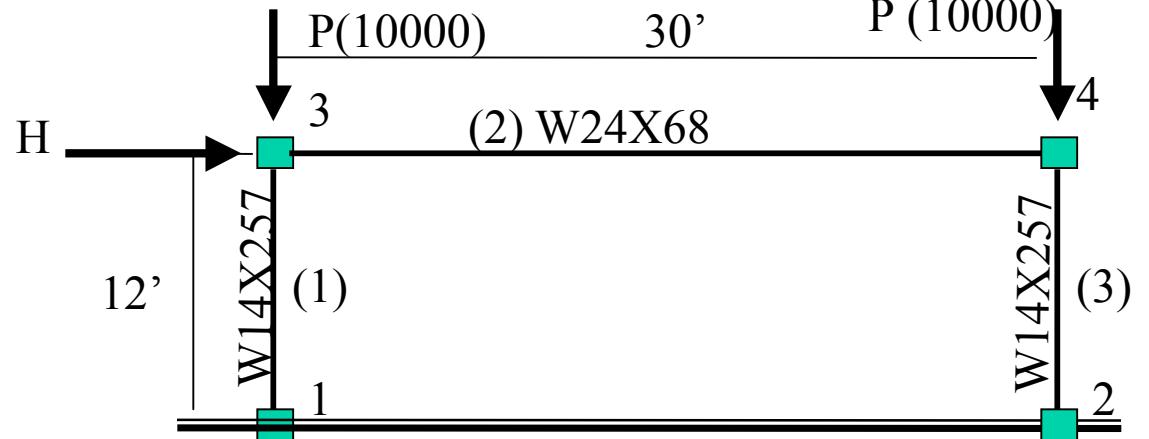
h.tcl

```
# create the model builder  
model Basic -ndm 2 -ndf 3  
  
# create 2 nodes  
node 1 0.0 0.0  
node 2 360.0 0.0  
node 3 0.0 144.0  
node 4 360.0 144.0  
  
# fix node 1  
fix 1 1 1 1  
fix 2 1 1 1
```

## geomTransf PDelta 1

```
geomTransf Linear 2  
  
# create elements  
source SteelWSections.tcl  
set in 1.0  
set E 30000  
ElasticBeamWSection2d 1 1 3 W14X257 $E 1  
ElasticBeamWSection2d 2 3 4 W24X68 $E 2  
ElasticBeamWSection2d 3 2 4 W14X257 $E 1
```

```
# create time series and load pattern  
set P 10000.0  
timeSeries Constant 1  
pattern Plain 1 1 {  
    load 3 0.0 -$P 0.0  
    load 4 0.0 -$P 0.0  
}
```



```
# create an analysis  
constraints Plain  
numberer RCM  
test NormDispIncr 1.0e-12 6 0  
algorithm Newton  
system ProfileSPD  
integrator LoadControl 1.0  
analysis Static  
  
# perform the analysis  
analyze 1  
  
timeSeries Linear 2  
pattern Plain 2 2 {  
    load 3 1.0 0.0 0.0  
    load 4 1.0 0.0 0.0  
}  
integrator DisplacementControl 3 1 0.1  
analyze 100; print node 3
```

Terminal — bash — 101x22

```

fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ OpenSees h.tcl

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Pacific Earthquake Engineering Research Center -- 2.3.0.alpha

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element elasticBeamColumn 1 1 3 75.6 30000 3400.0 1
element elasticBeamColumn 2 3 4 20.1 30000 1830.0 2
element elasticBeamColumn 3 2 4 75.6 30000 3400.0 1

Node: 3
    Coordinates : 0 144
    Disps: 10 -0.609521 -0.0787607
    unbalanced Load: 1080.3 -10000 0
    ID : 3 4 5

fmk:~/Desktop/Workshops/ChinaWorkshop2011/examples$ 
```

Displacement (x)	Load (blue line, PDelta)	Load (green line)
0	0	0
1	~340	~340
2	~680	~680
3	~1020	~1020
4	~1360	~1360
5	~1700	~1700
6	~2040	~2040
7	~2380	~2380
8	~2720	~2720
9	~3060	~3060
10	3400	2100

# Simply Supported Beam

n.tcl

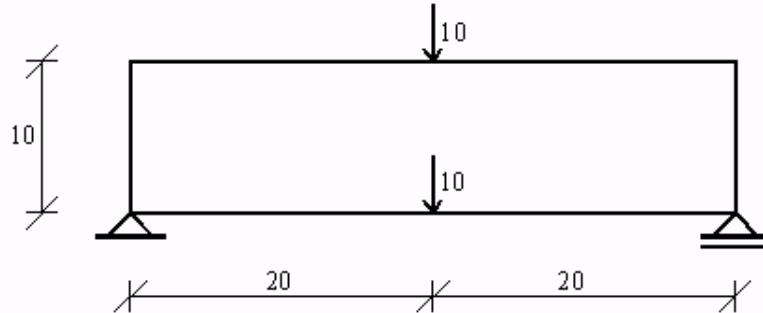


Fig. 1 Geometry and static loads

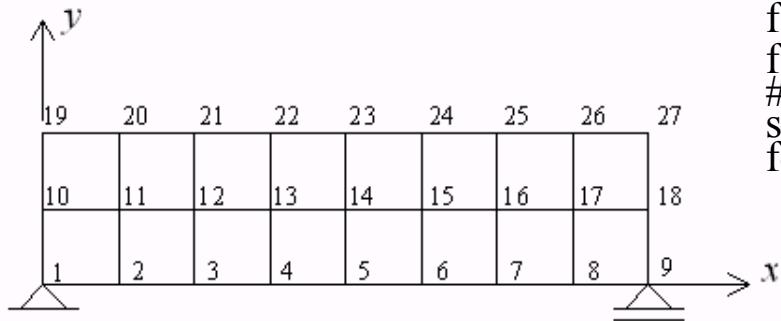


Fig. 2 Finite element mesh and node numbering

```
# some problem parameters
set L 40.0
set H 10.0
set thick 2.0
set P 10
set nX 9; # numNodes x dirn
set nY 3; # numNodes y dirn

# model builder
model Basic -ndm 2 -ndf 2

# create material
nDMaterial ElasticIsotropic 1 1000 0.25 3.0
```

```
# create nodes
set nodeTag 1
set yLoc 0.0;
for {set i 0} {$i < $nY} {incr i 1} {
    set xLoc 0.0;
    for {set j 0} {$j < $nX} {incr j 1} {
        node $nodeTag $xLoc $yLoc
        set xLoc [expr $xLoc+ $L/($nX-1.0)]
        incr nodeTag
    }
    set yLoc [expr $yLoc+ $H/($nY-1.0)]
}
# boundary conditions
fix 1 1 1
fix $nX 1 1
# create elements
set eleTag 1
for {set i 1} {$i < $nY} {incr i 1} {
    set iNode [expr 1+($i-1)*$nX];
    set jNode [expr $iNode+1];
    set kNode [expr $jNode+$nX]
    set lNode [expr $iNode+$nX]
    for {set j 1} {$j < $nX} {incr j 1}
        element quad $eleTag $iNode $jNode $kNode $lNode \
                    $thick "PlaneStress" 1
        incr eleTag; incr iNode; incr jNode; incr kNode; incr lNode
    }
}
# apply loads
set midNode [expr ($nX+1)/2]
timeSeries Linear 1
pattern Plain 1 1 {
    load $midNode 0 -$P
    load [expr $midNode + $nX*($nY-1)] 0 -$P
}
analysis Static;
analyze 1; print node $midNode
```

# Simply Supported Beam

o.tcl

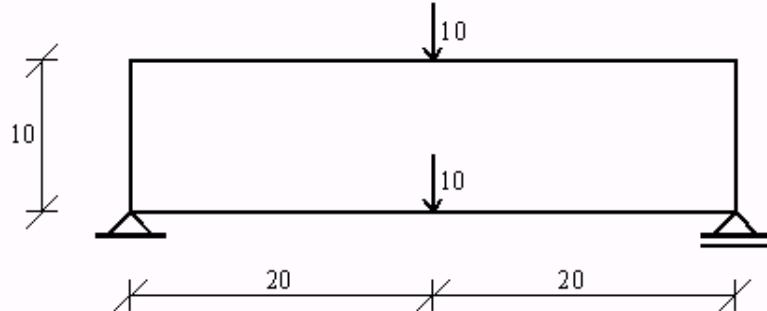


Fig. 1 Geometry and static loads

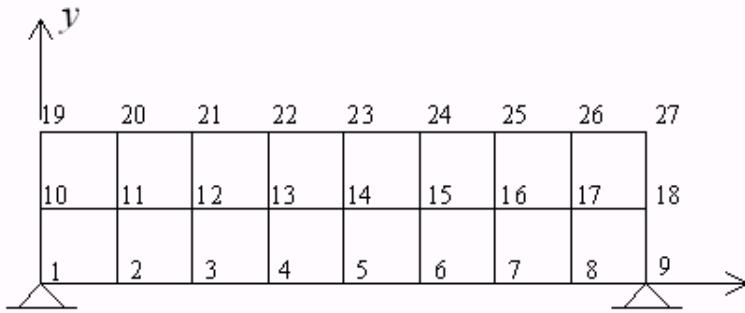


Fig. 2 Finite element mesh and node numbering

```
# some problem parameters
set L 40.0
set H 10.0
set thick 2.0
set P 10
set nX 9; # numNodes x dirn
set nY 3; # numNodes y dirn
```

```
# model builder
model Basic -ndm 2 -ndf 2
# create material
nDMaterial ElasticIsotropic 1 1000 0.25 3.0
```

```
# use block command
set cmd "block2D [expr $nX-1] [expr $nY-1] 1 1 \
quad \" $thick PlaneStress 1\" {
    1 0 0
    2 $L 0
    3 $L $H
    4 0 $H
}"
```

eval \$cmd

```
# apply loads
set midNode [expr ($nX+1)/2]
timeSeries Linear 1
pattern Plain 1 1 {
    load $midNode 0 -$P
    load [expr $midNode + $nX*($nY-1)] 0 -$P
}
analysis Static;
analyze 1;
print node $midNode
```

```
Terminal — bash — 85x37
examples> OpenSees n.tcl

    OpenSees -- Open System For Earthquake Engineering Simulation
    Pacific Earthquake Engineering Research Center -- 2.3.0.alpha

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            All Rights Reserved
        (Copyright and Disclaimer @ http://www.berkeley.edu/OpenSees/copyright.html)

Node: 5
    Coordinates : 20 0
    Disps: -1.37853e-16 -0.096041
    unbalanced Load: 0 -10
    ID : 26 27

examples> OpenSees o.tcl

    OpenSees -- Open System For Earthquake Engineering Simulation
    Pacific Earthquake Engineering Research Center -- 2.3.0.alpha

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Node: 5
    Coordinates : 20 0
    Disps: -1.37853e-16 -0.096041
    unbalanced Load: 0 -10
    ID : 26 27

examples> 
```

# Cantilevered Circular Column

p.tcl

```

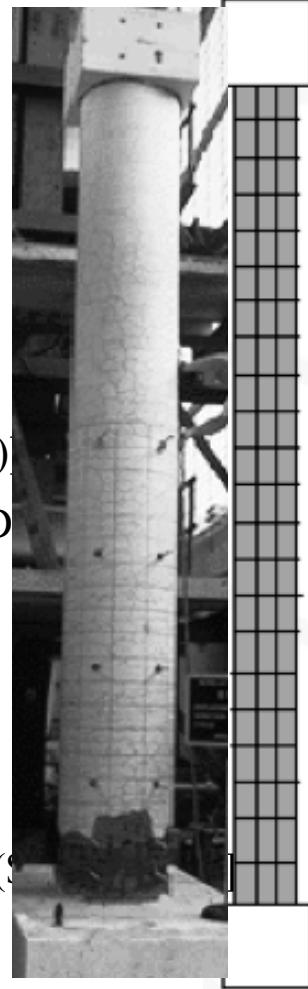
set P 1.0
set L 20.0
set R 1.0
set E 1000.0
set nz 20
set nx 6
set ny 6
set PI [expr 2.0 * asin(1.0)]
set I [expr $PI*pow((2*$R),4)/64.0]
puts "PL^3/3EI = [expr $P*pow($L,3)/(3.0*$E*$I)]"
# Create ModelBuilder with 3 dimensions and 6 D
model Basic -ndm 3 -ndf 3

# create the material
nDMaterial ElasticIsotropic 1 $E 0.25 1.27

set eleArgs "1"
set element bbarBrick

set nn [expr ($nz)*($nx+1)*($ny+1) + (($nx+1)*($ny+1)+1)]
set n1 [expr ($nz)*($nx+1)*($ny+1)+$nx]

```



```

# mesh generation
set sqrtR [expr sqrt($R/2.0)]
set cmd "$block3D $nx $ny $nz 1 1 $elc
1 -$sqrtR -$sqrtR 0
2 $sqrtR -$sqrtR 0
3 $sqrtR $sqrtR 0
4 -$sqrtR $sqrtR 0
5 -$sqrtR -$sqrtR $L
6 $sqrtR -$sqrtR $L
7 $sqrtR $sqrtR $L
8 -$sqrtR $sqrtR $L
13 0 -$R 0
14 $R 0 0
15 0 $R 0
16 -$R 0 0
18 0 -$R $L
19 $R 0 $L
20 0 $R $L
21 -$R 0 $L
23 0 -$R [expr $L/2.0]
24 $R 0 [expr $L/2.0]
25 0 $R [expr $L/2.0]
26 -$R 0 [expr $L/2.0]
}"

```

**eval \$cmd**

```
# boundary conditions
fixZ 0.0 1 1 1
```

```
# Constant point load
pattern Plain 1 Linear {
    load $nn 0.0 $P 0.0
}
```



Terminal — OpenSees — 79x22

examples> OpenSees

OpenSees -- Open System For Earthquake Engineering Simulation  
Pacific Earthquake Engineering Research Center -- 2.3.0.alpha

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)

OpenSees > source p.tcl  
PL^3/3EI = 3.3953054526271007

Node: 1005

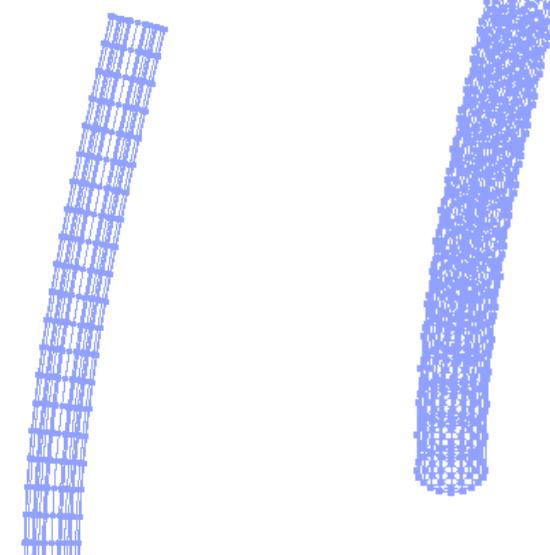
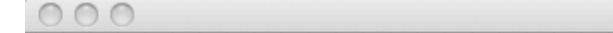
Coordinates : 0 0 20

Disps: 1.35333e-11 3.14833 1.24813e-14

unbalanced Load: 0 1 0

ID : 99 100 101

OpenSees > █



# Any Questions?