

# Geotechnical Elements and Models in **OpenSees**

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**OpenSees User Workshop, Monday August 22, 2011**



# Type of Geotechnical Problems that can be solved using **OpenSees**

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## ○ Static Problems

- Deformation analyses (1D, 2D, or 3D)
- Consolidation problems (diffusion problems)
- Soil-structure interaction problems
  - Shallow foundations (e.g. bearing capacity, settlements)
  - Pile foundations (e.g. vertical and lateral capacity)

## ○ Dynamic (earthquake problems)

- Free-field analysis
- Liquefaction induced problems
- Soil structure interaction problems (e.g. response of pile foundations, bridge bents, or complete structures embedded in soils to earthquake excitations)



# What do we need??

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- Solid **elements** to characterize the soil domain (continuum).
- Appropriate **boundary conditions** to accurately represent the soil domain boundaries.
- Robust **constitutive models** to characterize the soil stress-strain response under monotonic and cyclic loading conditions
- **Interface elements** to capture the interaction between the soil and adjacent structures.
- **Everything else** you are learning in this workshop (i.e., how to create beam elements, apply loads and boundary conditions, record results, perform the analysis, etc.



# Outline

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- **Finite Elements** (for solids)
  - Single-phase
  - Multi-phase (coupled) finite elements
  - Zero length element
- **Material Models**
  - Elastic
  - Elasto-plastic Continuum Models
  - Elasto-plastic Uniaxial models
- **Boundary Conditions**
  - Equal DOF
  - Absorbent boundaries



# Finite Elements (solids)

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- **Single-phase formulations**

- To capture the response of dry soils (or total stress analysis) → need one single phase
  - Phase 1 – soil skeleton

- **Multi-phase formulations**

- To capture the response of saturated soils (effective stress analysis) → need two phases
  - Phase 1 → Soil skeleton
  - Phase 2 → pore water

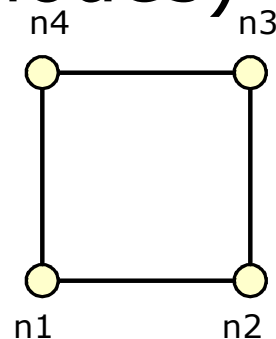
- **Zero-Length element**

- To capture interface response between solid and beam elements, and to apply absorbent boundary conditions

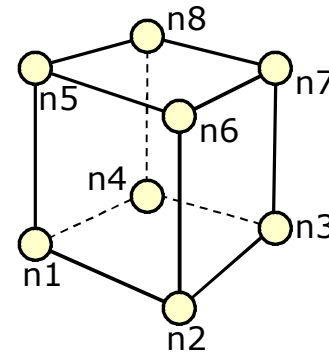
# Single Phase Formulations

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- Small deformation solid elements
  - 2-D quadrilateral elements (4, 9 nodes)
  - 3-D solid elements, brick (8, 20 nodes)
- Large deformation (total Lagrangian) solid elements, bricks (20 nodes)



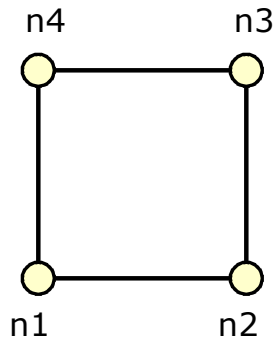
**quad** (4 node)



**stdBrick** (8 node)

# quad element definition

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**quad** (4 node)

```
element quad $eleTag $n1 $n2 $n3 $n4 $thick $type $matTag  
<$press $rho $b1 $b2>
```

Must define first all the required arguments. In particular:

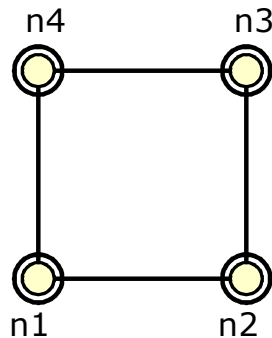
**Nodes** \$n1, \$n2, \$n3, \$n4 and

**Material type** \$matTag

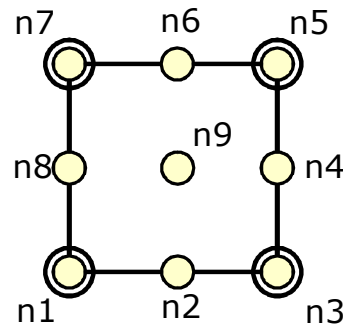
The arguments in <...> are optional

# Multi-Phase Formulations

- Fully coupled u-p elements (2D & 3D)
- Fully coupled u-p-U elements (3D) for small deformations



**quadUP**



**9\_4\_quadUP**

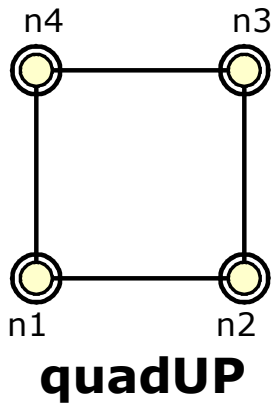
Degrees of Freedom (DOFs) are:

- u → solid displacement, on ○
- P → pore fluid pressures, on ○
- U → pore fluid displacements, on ○



# quadUP element definition

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```
element quadUP $eleTag $n1 $n2 $n3 $n4 $thick $type $matTag  
$bulk $fmass $hPerm $vPerm <$b1 $b2 $t>
```

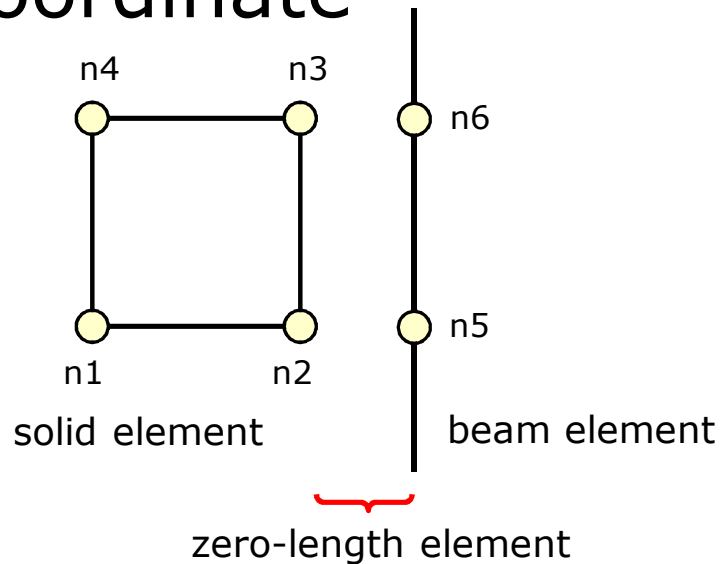
\$bulk → combined undrained bulk modulus  $B_c = B_f/n$

\$fmass → fluid mass density

\$hperm & \$vperm → horiz. And vert. permeability

# zerolength element

- Connects two points at the same coordinate



```
element zeroLength $eleTag $n1 $n2 -mat $matTag1 $matTag2 ...  
-dir $dir1 $dir2 ... <-orient $x1 $x2 $x3 $yp1 $yp2 $yp3>
```



# Material Models

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- Linear Elastic Material model (**nDMaterial**)
  - To characterize the response of the soil (or other continuum) in its elastic regime
- Elasto-Plastic Material models (**nDMaterial**)
  - To characterize the nonlinear stress-strain response of soils
- Elasto-plastic Uniaxial models
  - To characterize the interface response between soil and structural elements (**uniaxialMaterial**).



# nDMaterial Elastic

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- Small deformation elasticity
  - Linear isotropic
  - Nonlinear isotropic
  - Cross anisotropic
  
- Elastic Isotropic Material

**nDMaterial ElasticIsotropic**  $\text{\$matTag}$   $\text{\$E}$   $\text{\$v}$



# nDMaterial

## Elasto-Plastic (Small Deformations)

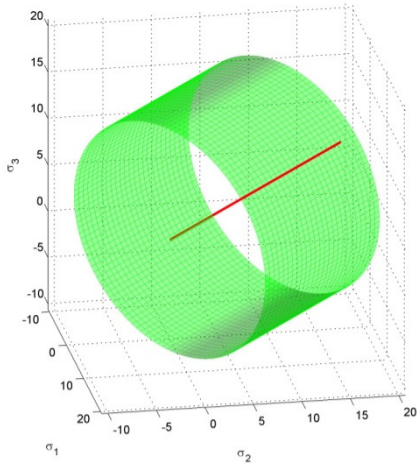
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- J2-Plasticity Material (von Mises)
- Drucker-Prager Material (UW)
- Template Elasto-Plastic Material (UC Davis)
- Cam-Clay Material (Berkeley, UW)
- MutiYield Materials (San Diego)
- FluidSolidPorous Material(SanDiego)

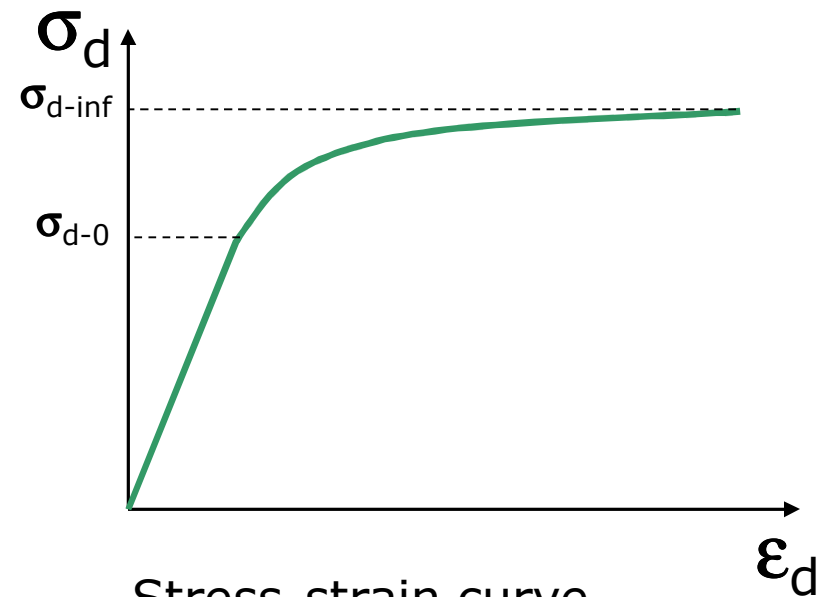
# nDMaterial J2Plasticity

○ von-Mises type

**nDMaterial J2Plasticity**  $\$matTag$   $\$K$   $\$G$   $\$sig0$   $\$sigInf$   $\$delta$   $\$H$



Von-Mises Yield Surface



Stress-strain curve



# nDMaterial

## Template Elasto-Plastic Material

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- Versatile tool to generate multiple types of elasto-plastic materials by combining **yield surfaces, plastic potentials and evolution laws**
- Developed by Boris Jeremic at UC Davis  
<http://sokocalo.engr.ucdavis.edu/~jeremic>

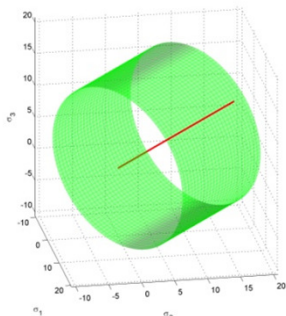
# nDMaterial

## Template Elasto-Plastic Material

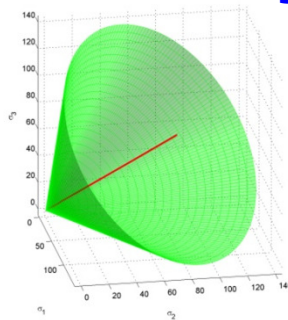
```
nDMaterial Template3Dep $matTag $ElmatTag  
-YS $ys -PS $ps -EPS $eps <-ELS1 $el> <-ELT1 $et>
```

- YS** → **Yield surfaces** (von Mises, Drucker Prager, Mohr-Coulomb, Camclay)
- PS** → **Plastic potentials** (von Mises, Drucker-Prager Mohr-Coulomb, Camclay, Leon)
- EPS** → **Initial state of stress**
- ELS1** → **Scalar evolution laws** for isotropic hardening (linear, nonlinear Camclay)
- ELT1** → **Tensorial evolution laws** for Kinematic hardening (linear, nonlinear Armstrong-Frederick)

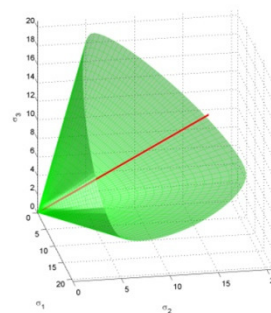
**Von-Mises**



**Drucker Prager**



**Mohr-Coulomb**



**Camclay**







# nDMaterial MultiYield Materials

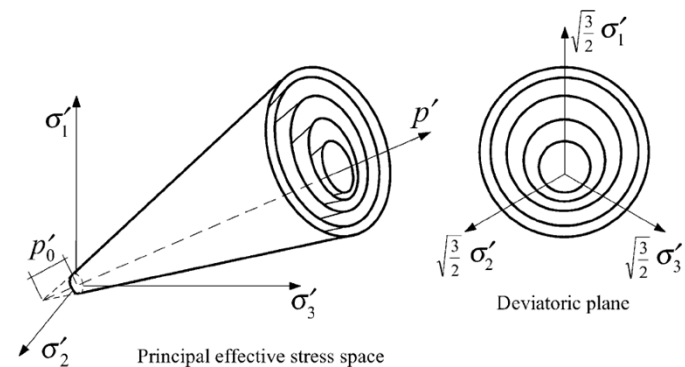
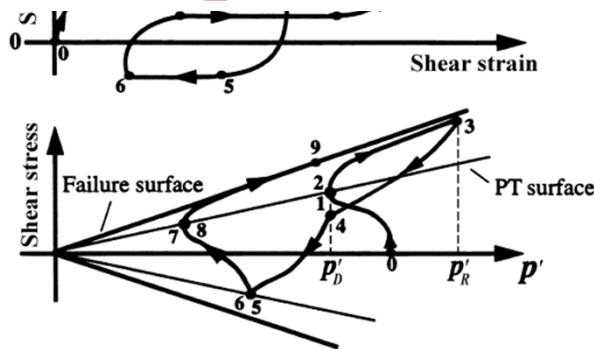
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- Material models based on Multiyield Plasticity (*Mroz et al., Prevost et al.*)
- Two types
  - Pressure Independent Multi-yield (for total stress analysis)
  - Pressure Dependent Multi-yield (captures well the response of liquefiable soils)
  - Fluid-solid porous material (Material to couple solid & fluid phases)
- Developed by Elgamal et al. at UCSD  
<http://cyclic.ucsd.edu/opensees/>

# nDMaterial PressureDependentMultiYield

**nDMaterial PressureDependMultiYield**  $\$matTag$   $\$nd$   $\$rho$   
 $\$refShearModul$   $\$refBulkModul$   $\$frictionAng$   $\$peakShearStra$   
 $\$refPress$   $\$pressDependCoe$   $\$PTAng$   
 $\$contrac$   $\$dilat1$   $\$dilat2$ ,  $\$liquefac1$   $\$liquefac2$   $\$liquefac3$   
 $\langle \$noYieldSurf=20 \langle \$r1 \ \$Gs1 \ \dots \rangle$   
 $\$e=0.6$   $\$cs1=0.9$   $\$cs2=0.02$   $\$cs3=0.7$   $\$pa=101 \rangle$

## 15 parameters!!!???



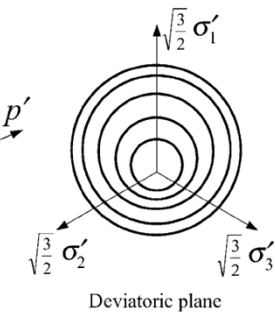
# nDMaterial PressureDependentMultiYield

nDMat  
\$refShe  
\$refPre  
\$contra  
<\$noYi  
\$e=0.

	Loose Sand (15%-35%)	Medium Sand (35%-65%)	Medium-dense Sand (65%-85%)	Dense Sand (85%-100%)
<i>rho</i> (ton/m <sup>3</sup> )	1.7	1.9	2.0	2.1
<i>refShearModul</i> (kPa, at $p'_r=80$ kPa)	$5.5 \times 10^4$	$7.5 \times 10^4$	$1.0 \times 10^5$	$1.3 \times 10^5$
<i>refBulkModu</i> (kPa, at $p'_r=80$ kPa)	$1.5 \times 10^5$	$2.0 \times 10^5$	$3.0 \times 10^5$	$3.9 \times 10^5$
<i>frictionAng</i>	29	33	37	40
<i>peakShearStra</i> (at $p'_r=80$ kPa)	0.1	0.1	0.1	0.1
<i>refPress</i> ( $p'_r$ , kPa)	80	80	80	80
<i>pressDependCoe</i>	0.5	0.5	0.5	0.5
<i>PTAng</i>	29	27	27	27
<i>contrac</i>	0.21	0.07	0.05	0.03
<i>dilat1</i>	0.	0.4	0.6	0.8
<i>dilat2</i>	0	2	3	5
<i>liquefac1</i> (kPa)	10	10	5	0
<i>liquefac2</i>	0.02	0.01	0.003	0
<i>liquefac3</i>	1	1	1	0
<i>e</i>	0.85	0.7	0.55	0.45

nd \$rho  
earStra  
fac3

Shear stress  
Shear stress



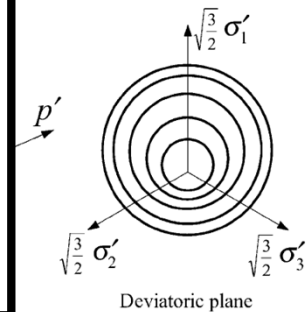
# nDMaterial PressureDependentMultiYield02

nDMaterial  
 \$refBulkMod  
 \$pressDependCoe  
 \$contrac1  
 \$noYield  
 \$contrac2  
 \$e=0.6 \$

	Dr=30%	Dr=40%	Dr=50%	Dr=60%	Dr=75%
<i>rho</i> (ton/m3)	1.7	1.8	1.9	2.0	2.1
<i>refShearModul</i> (kPa, at $p'_i=80$ kPa)	$6 \times 10^4$	$9 \times 10^4$	$10 \times 10^4$	$11 \times 10^4$	$13 \times 10^4$
<i>refBulkModu</i> (kPa, at $p'_i=80$ kPa)	$16 \times 10^4$	$22 \times 10^4$	$23.3 \times 10^4$	$24 \times 10^4$	$26 \times 10^4$
	( $K_0=0.5$ )	( $K_0=0.47$ )	( $K_0=0.45$ )	( $K_0=0.43$ )	( $K_0=0.4$ )
<i>frictionAng</i>	31	32	33.5	35	36.5
<i>PTAng</i>	31	26	25.5	26	26
<i>peakShearStra</i> (at $p'_i=101$ kPa)	0.1				
<i>refPress</i> ( $p'_i$ , kPa)	101				
<i>pressDependCoe</i>	0.5				
<i>Contrac1</i>	0.087	0.067	0.045	0.028	0.013
<i>Contrac3</i>	0.18	0.23	0.15	0.05	0.0
<i>dilat1</i>	0.	0.06	0.06	0.1	0.3
<i>dilat3</i>	0.0	0.27	0.15	0.05	0.0
<i>e</i>	0.85	0.77	0.7	0.65	0.55

\$nd \$rho

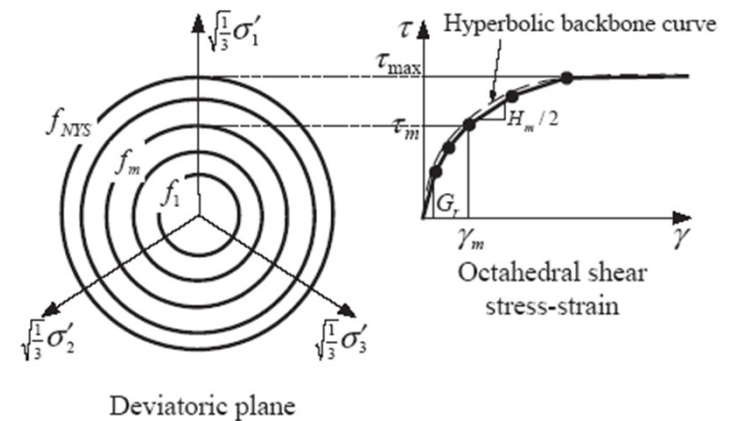
= 0.0



$\sigma'_2$  Principal effective stress space

# nDMaterial PressureIndependentMultiYield

**nDMaterial PressureIndependMultiYield** \$matTag \$nd \$rho  
 \$refShearModul \$refBulkModul \$cohesi \$peakShearStra  
 \$frictionAng \$refPress=101 \$pressDependCoe=0.  
 <\$noYieldSurf=20 <\$r1 \$Gs1 ...>>





# nDMaterial FluidSolidPorousMaterial

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- Couples the response of two phases: fluid and solid – developed to simulate the response of saturated porous media

**nDMaterial FluidSolidPorousMaterial** \$matTag \$nd  
\$soilMatTag \$combinedBulkModul

\$soilMatTag → the tag of previously defined material  
\$combinedBulkModul → combined undrained bulk modulus,  
 $B_c = B_f/n$



# nDMaterial FluidSolidPorousMaterial

---

- Couples the response of two phases: fluid and solid – developed to simulate the response of saturated porous media

**nDMaterial FluidSolidPorousMaterial** \$matTag \$nd  
\$soilMatTag \$combinedBulkModul

\$soilMatTag → the tag of previously defined material  
\$combinedBulkModul → combined undrained bulk modulus,  
 $B_c = B_f/n$



# nDMaterial

## Other Models under development

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**nDMaterial BoundingCamClay**

**nDMaterial Manzari-Dafalias**





## Additional commands for **multiyield** materials

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- Help perform stage analysis

**updateMaterialStage** **-material** \$matTag **-stage** \$sNum

\$MatTag → the tag of previously defined material

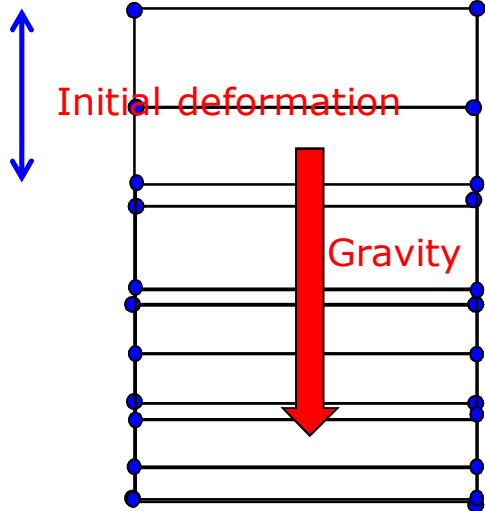
\$sNum → (0 - elastic, 1-plastic, 2 – linear elastic constant **f( $\sigma_3$ )** )

**updateParameter** **-material** \$matTag **-refG** \$newVal

\$MatTag → the tag of previously defined material

\$sNewVal → new parameter value

# Initial State for Geotechnical Problems



```
# turn on initial state analysis feature
InitialStateAnalysis on

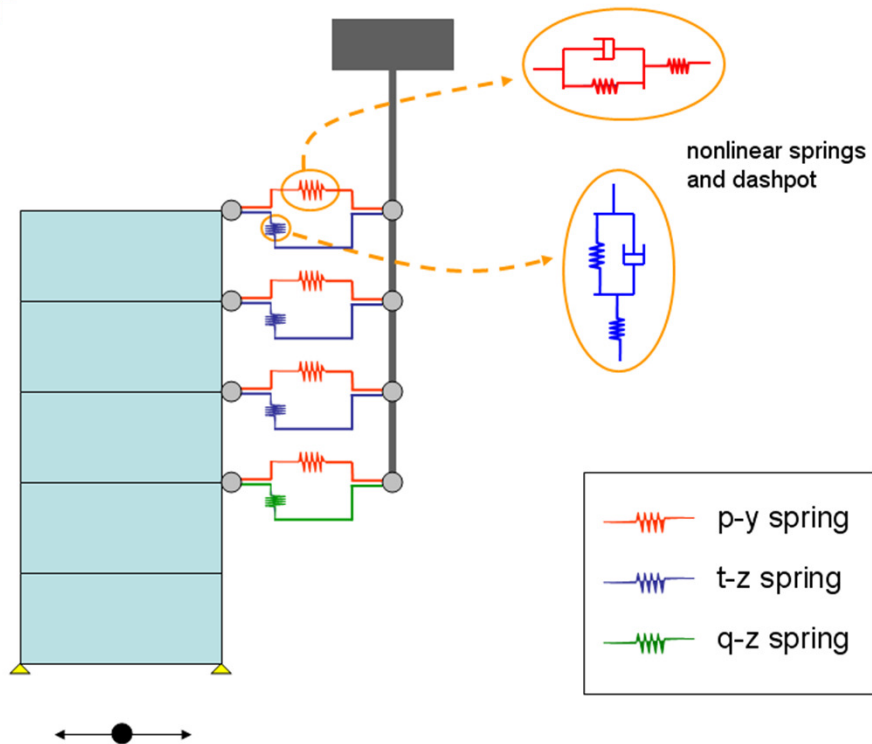
# create incremental gravity load
pattern Plain 3 {Series -time {0 10 10000} -values {0 1 1} -factor 1} {
  eleLoad -ele 1 -type -selfWeight
  eleLoad -ele 2 -type -selfWeight
  .
  .
}

analysis steps ...

# turn on initial state analysis feature
InitialStateAnalysis off
```

# Elasto-plastic Uniaxial models

- To capture interface response between solid (soil) and beam elements (pile)



Py Tz Qz Uniaxial Materials

- PySimple1
- TzSimple1
- QzSimple1
  
- PyLiq1
- TzLiq1

# uniaxialMaterial PySimple1

```
uniaxialMaterial PySimple1 matTag $soilType $pult $Y50 $Cd  
<$c>
```

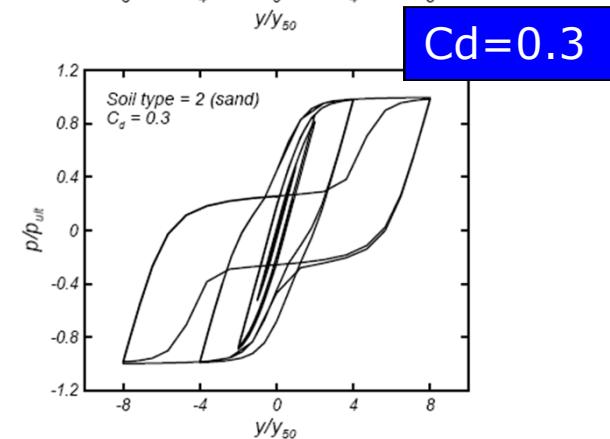
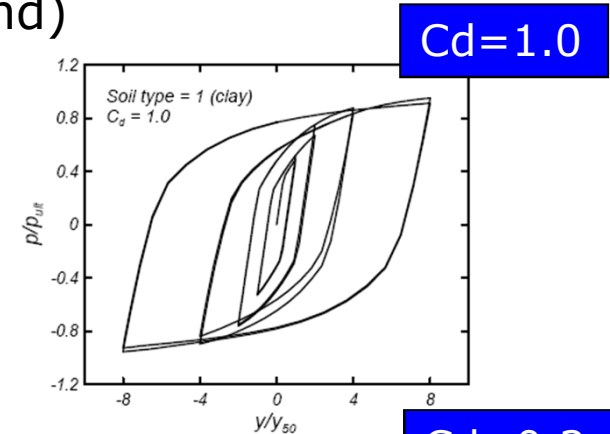
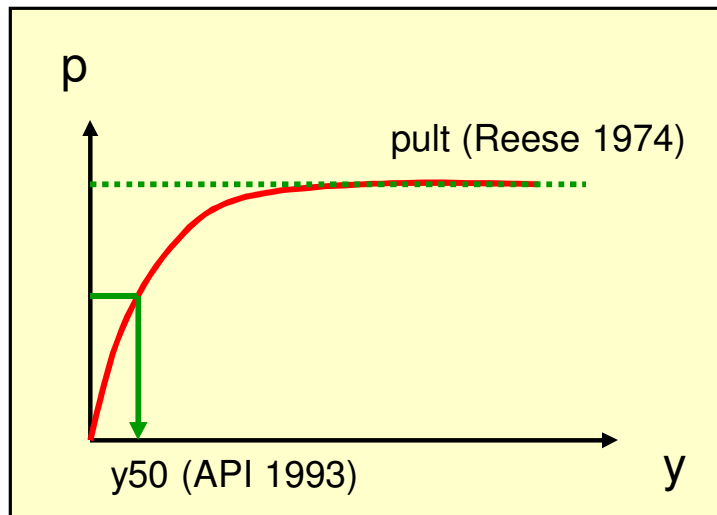
\$soilType → =1 Matlock (clay), =2 API (sand)

\$pult → ultimate capacity of p-y material

\$Y50 → displ. @ 50% of pult

Cd → drag resistance (=1 no gap, <1 gap)

\$c → viscous damping





# uniaxialMaterial TzSimple1 & QzSimple1

```
uniaxialMaterial TzSimple1 matTag $tzType $tult $z50 <$c>
```

\$tzType → =1 Reese & O'Neill (clay), =2 Mosher (sand)

\$tult → ultimate capacity of t-z material

\$z50 → displ. @ 50% of tult

\$c → viscous damping

```
uniaxialMaterial QzSimple1 matTag $qzType $qult $z50  
<$suction $c>
```

\$qzType → =1 Reese & O'Neill (clay), =2 Vijayvergiya (sand)

\$qult = ultimate capacity of q-z material

\$z50 = displ. @ 50% of qult

\$suction → uplift resistance = suction\*qult

\$c viscous damping



# uniaxialMaterial PyLiq1

```
uniaxialMaterial PyLiq1 $matTag $soilType $pult $Y50 $Cd $c  
$pRes $solidElem1 $solidElem2
```

\$soilType → =1 Matlock (clay), =2 API (sand)

\$pult → ultimate capacity of p-y material

\$Y50 → displ. @ 50% of pult

Cd → drag resistance (=1 no gap, <1 gap)

\$c → viscous damping

\$pRes → residual (minimum) p-y resistance as  $r_u=1.0$

\$solidElem1 & \$solidElem2 → solid elements from which PyLiq1 will obtain effective stresses and pore pressures

# uniaxialMaterial PyLiq1

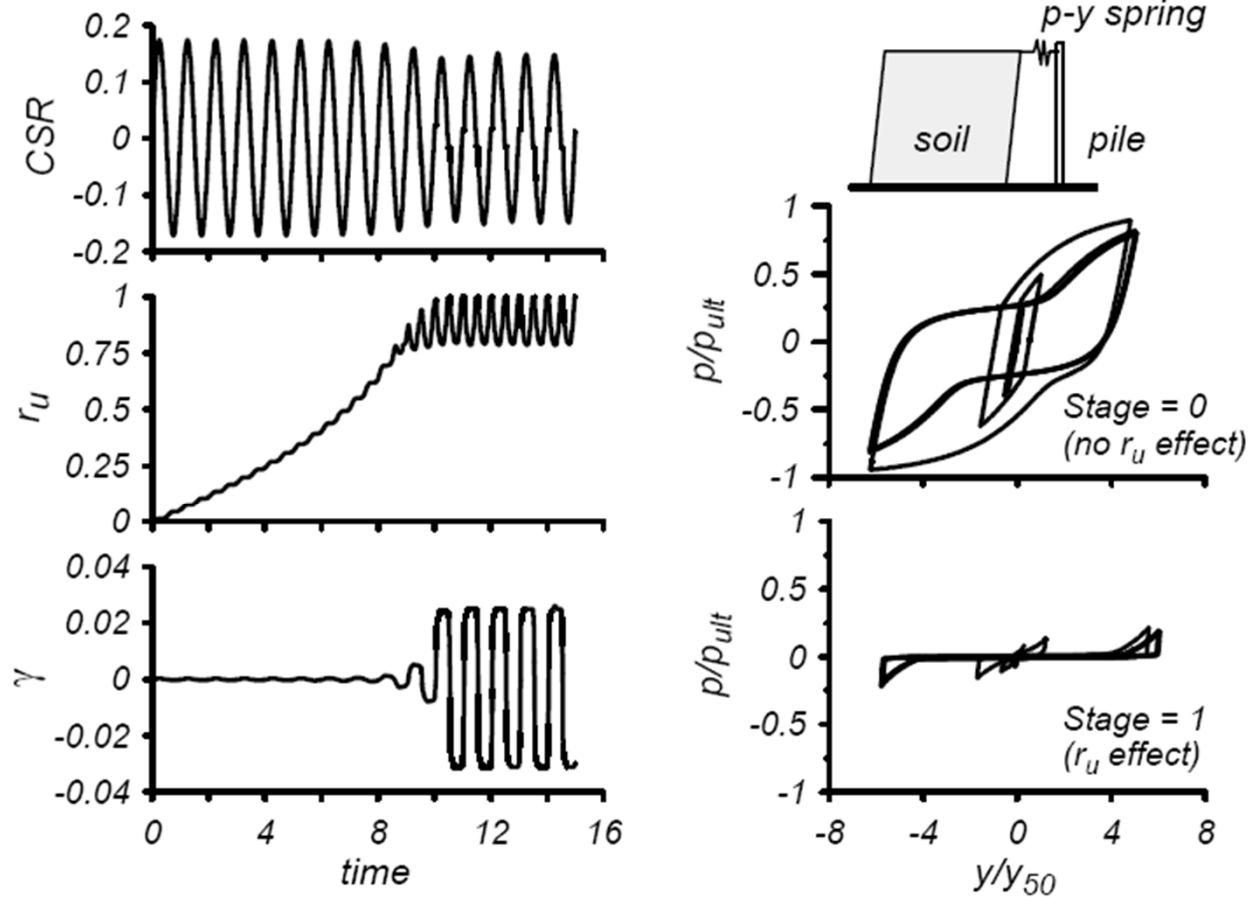


FIG. Example of PyLiq1 behavior during liquefaction without lateral spreading.

# Boundary Conditions

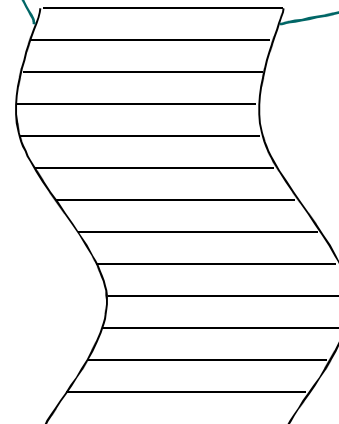
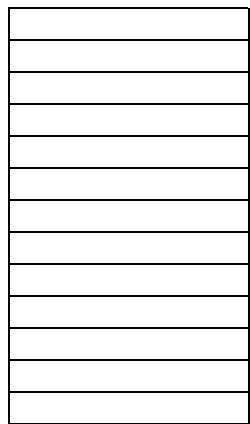
## ○ EqualDof

```
equalDOF $rNodeTag $cNodeTag $dof1 $dof2 ...
```

\$rNodeTag → master node

\$cNodeTag → slave node

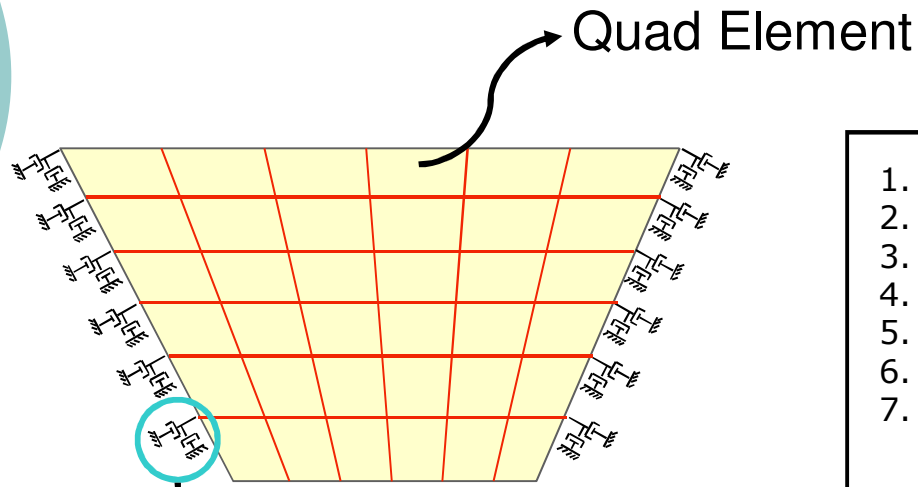
\$dof1 \$dof2 ... → constrained dof's



Same lateral deformation

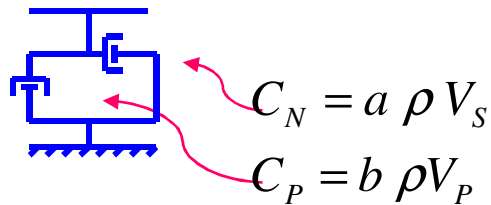


# Absorbent/transmitting Boundaries Lysmer (1969)



```

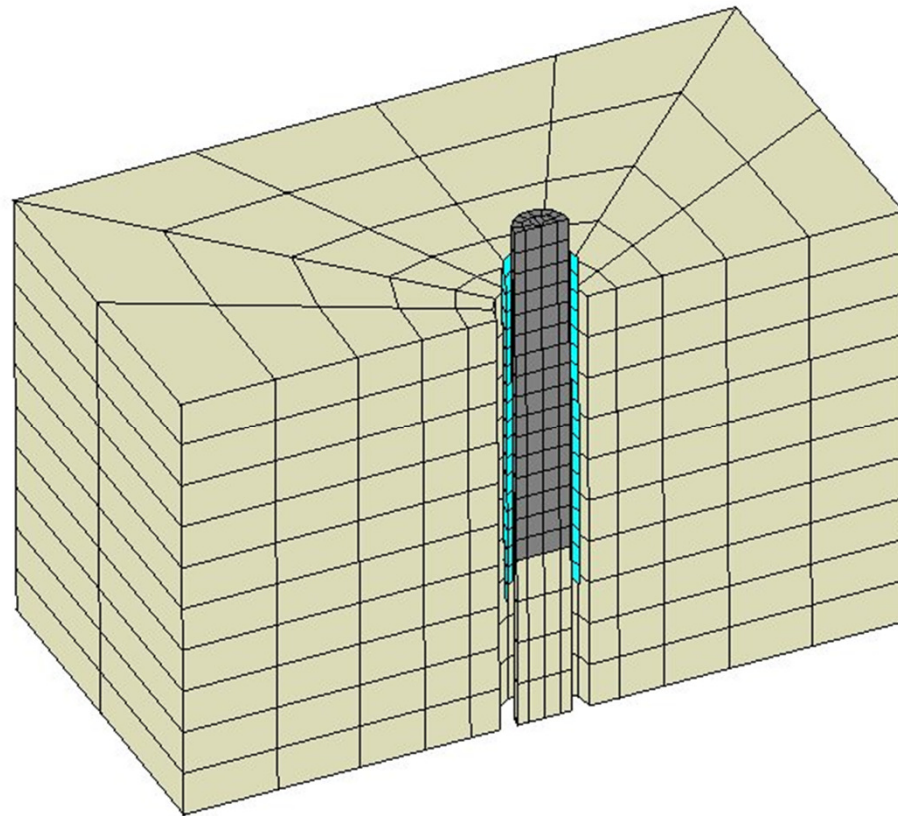
1. set DampP 755
2. set DampN 1216
3. uniaxialMaterial Elastic 1 0 $DampP
4. uniaxialMaterial Elastic 2 0 $DampN
5. node 1 16.0 0.0
6. node 2 16.0 0.0
7. element zeroLength 1 1 2 -mat 1 2
   -dir 1 2 -orient 1 -2 0 2 1 0
  
```



zeroLength Element &  
uniaxial material

# Contact Elements available in OpenSees

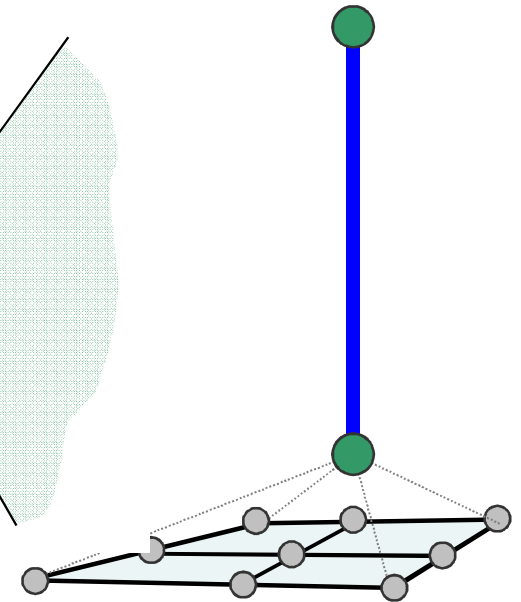
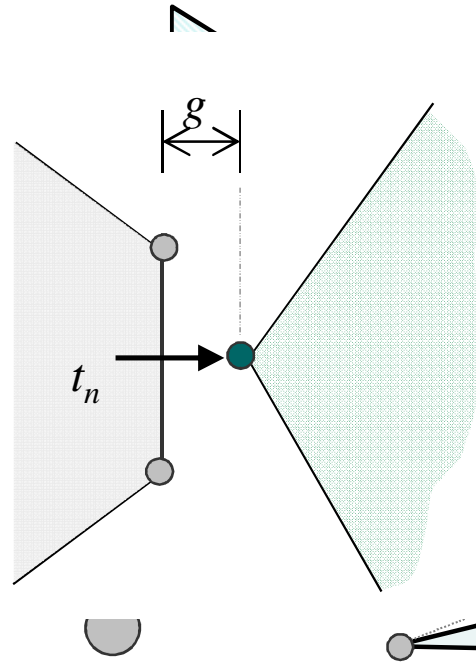
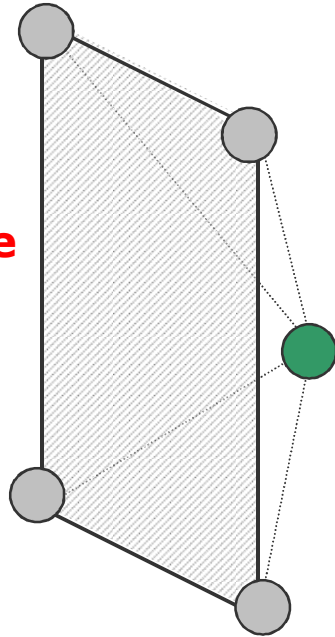
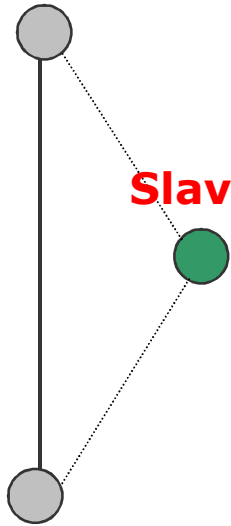
---



# Contact Elements available in OpenSees

Master node

Slave node



2D Node-to-Line Element

3D Node-to-Surface Element

3D Beam-to-Solid Element

3D End-Beam-to-Solid Element

# Contact Elements available in OpenSees

```
element SimpleContact2D $eleTag $iNode $jNode $sNode  
$lNode $matTag $gTol $fTol
```

\$eleTag → unique integer tag identifying element object

\$iNode \$jNode → master nodes

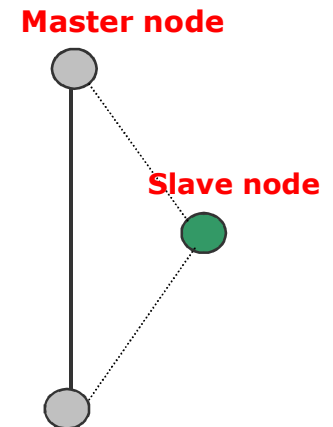
\$sNode → slave node

\$lNode → Lagrange multiplier node

\$matTag → unique integer tag associated with previously-defined nDMaterial object

\$gTol → gap tolerance

\$fTol → force tolerance



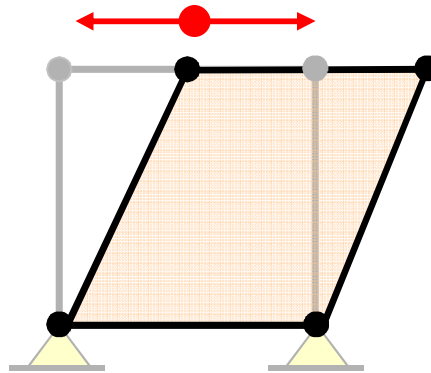


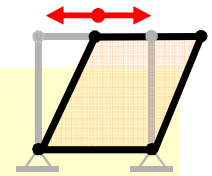
Many more capabilities currently  
under development!!

# Basic Example

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- Response of saturated soil element to harmonic excitation





```

#Created by Zhaohui Yang (zhyang@ucsd.edu)
#plastic pressure dependent material
#plane strain, single element, dynamic analysis (input motion: sinusoidal acceleration at base)
#SI units (m, s, KN, ton)
#
#      4   3
#      ----
#      |   |
#      |   |
#      |   |
#      1-----2 (nodes 1 and 2 fixed)
#      ^     ^
#      <--> input motion: sinusoidal acceleration at base
wipe
#

```

## Define basic variables using good names!!!

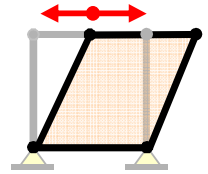
### #some user defined variables

```

#
set accMul 2      ;# acceleration multiplier
set massDen 2.0   ;# solid mass density
set fluidDen 1.0  ;# fluid mass density
set massProportionalDamping 0.0 ;
set stiffnessProportionalDamping 0.001 ;
set fangle 31.40  ;#friction angle
set ptangle 26.50 ;#phase transformation angle
set E 90000.0     ;#shear modulus
set poisson 0.40 ;
set G [expr $E/(2*(1+$poisson))];
set B [expr $E/(3*(1-2*$poisson))];
set press 0.0 ;# isotropic consolidation pressure on quad element(s)
set deltaT 0.010 ;# time step for analysis
set numSteps 2000 ;# Number of analysis steps
set gamma 0.600 ;# Newmark integration parameter
set period 1 ;# Period of applied sinusoidal load
set pi 3.1415926535 ;
set inclination 0 ;
set unitWeightX [expr ($massDen-$fluidDen)*9.81*sin($inclination/180.0*$pi)] ;# unit weight in X direction
set unitWeightY [expr -($massDen-$fluidDen)*9.81*cos($inclination/180.0*$pi)] ;# unit weight in Y direction

```

# Define model geometry, materials & fixities



```
#####  
#####
```

```
#create the ModelBuilder  
model basic -ndm 2 -ndf 2
```

```
# define material and properties  
nDMaterial PressureDependMultiYield 2 2 $massDen $G $B $fangle .1 80 0.5 \  
    $ptangle 0.17 0.4 10 10 0.015 1.0  
nDMaterial FluidSolidPorous 1 2 2 2.2D+6  
  
updateMaterialStage -material 1 -stage 0  
updateMaterialStage -material 2 -stage 0
```

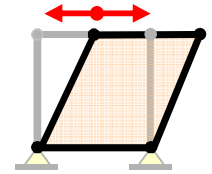
```
# define the nodes  
node 1 0.0D0 0.0D0  
node 2 1.0D0 0.0D0  
node 3 1.0D0 1.0D0  
node 4 0.0D0 1.0D0
```

```
# define the element thick material maTag press mDensity gravity  
element quad 1 1 2 3 4 1.0 "PlaneStrain" 1 $press 0. $unitWeightX $unitWeightY
```

```
# fix the base  
fix 1 1 1  
fix 2 1 1
```

```
#tie nodes 3 and 4  
equalDOF 3 4 1 2
```





# Define gravity step

## VERY IMPORTANT!!!

```
#####  
# GRAVITY APPLICATION (elastic behavior)
```

```
# create the SOE, ConstraintHandler, Integrator, Algorithm and Numberer  
system ProfileSPD  
test NormDispIncr 1.D-12 25 0  
constraints Transformation  
integrator LoadControl 1 1 1 1  
algorithm Newton  
numberer RCM
```

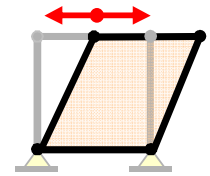
```
# create the Analysis  
analysis Static
```

```
#analyze  
analyze 2
```

```
# switch the material to plastic  
updateMaterialStage -material 1 -stage 1  
updateMaterialStage -material 2 -stage 1  
updateParameter -material 2 -refB [expr $G*2/3.];
```

```
#analyze  
analyze 1
```

# Define dynamic step



```
#####  
# NOW APPLY LOADING SEQUENCE AND ANALYZE (plastic)
```

```
# rezero time  
setTime 0.0  
wipeAnalysis
```

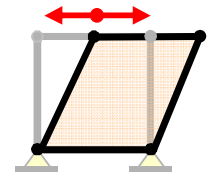
```
#create loading pattern  
pattern UniformExcitation 1 1 -accel "Sine 0 1000 $period -factor $accMul"
```

```
# create the Analysis  
constraints Transformation;  
test NormDispIncr 1.D-12 25 0  
algorithm Newton  
numberer RCM  
system ProfileSPD  
integrator Newmark $gamma [expr pow($gamma+0.5, 2)/4] \  
    $massProportionalDamping 0.0 $stiffnessProportionalDamping 0.0  
analysis VariableTransient
```

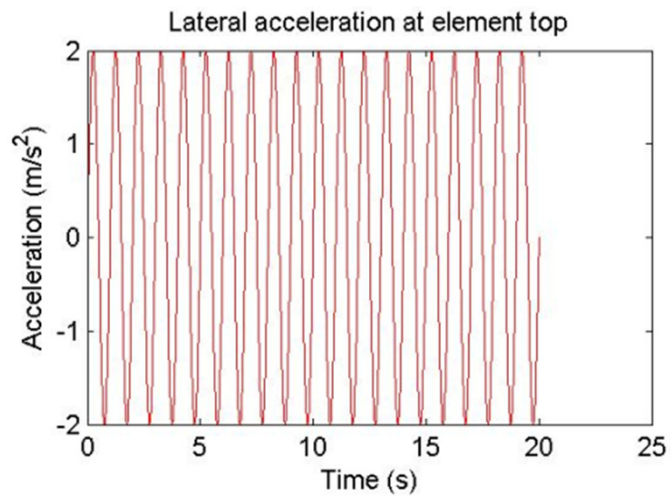
```
#create Recorders  
recorder Node -file disp.out -time -node 1 2 3 4 -dof 1 2 -dT 0.01 disp  
recorder Node -file acce.out -time -node 1 2 3 4 -dof 1 2 -dT 0.01 accel  
recorder Element -ele 1 -time -file stress1.out material 1 stress -dT 0.01  
recorder Element -ele 1 -time -file strain1.out material 1 strain -dT 0.01  
recorder Element -ele 1 -time -file stress3.out material 3 stress -dT 0.01  
recorder Element -ele 1 -time -file strain3.out material 3 strain -dT 0.01  
recorder Element -ele 1 -time -file press1.out material 1 pressure -dT 0.01  
recorder Element -ele 1 -time -file press3.out material 3 pressure -dT 0.01
```

```
#analyze  
set startT [clock seconds]  
analyze $numSteps $deltaT [expr $deltaT/100] $deltaT 10  
set endT [clock seconds]  
puts "Execution time: [expr $endT-$startT] seconds."  
  
wipe #flush output stream
```

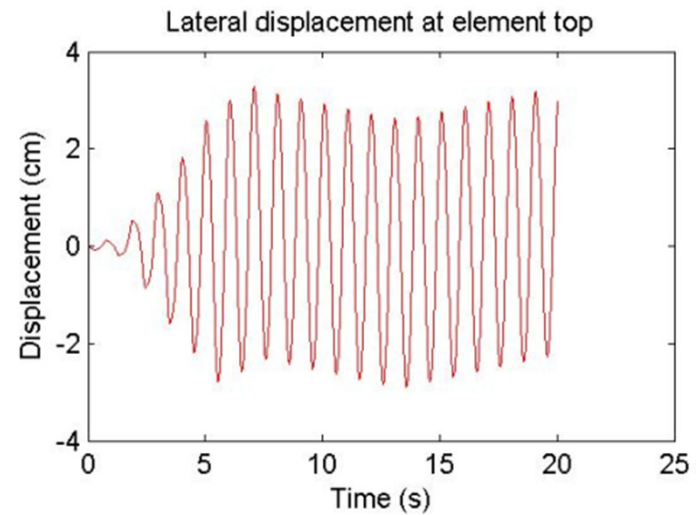
# Plot Results



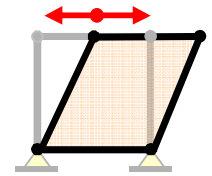
## Input Accel time history



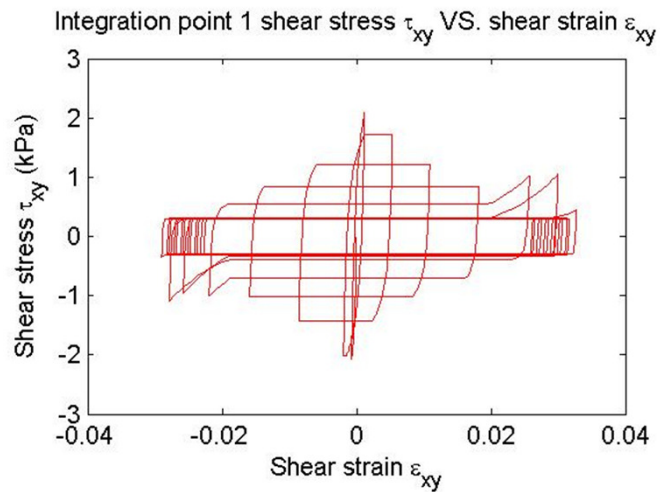
## Output displ. time history



# Plot results



## Stress-strain & stress path



## Pore pressures

