



# Geotechnical Elements and Models in **OpenSees**

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## Type of Geotechnical Problems that can be solved using **OpenSees**

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- Static Problems
  - Deformation analyses (1D, 2D, or 3D)
  - Consolidation problems (diffusion problems)
  - Soil-structure interaction problems
    - Shallow foundations (e.g. bearing capacity, settlements)
    - Pile foundations (e.g. vertical and lateral capacity)
  
- Dynamic (earthquake problems)
  - Free-field analysis
  - Liquefaction induced problems
  - Soil structure interaction problems (e.g. response of pile foundations, bridge bents, or complete structures embedded in soils to earthquake excitations)



## What do we need??

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- Solid **elements** to characterize the soil domain (continuum).
- Appropriate **boundary conditions** to accurately represent the soil domain boundaries.
- Robust **constitutive models** to characterize the soil stress-strain response under monotonic and cyclic loading conditions
- **Interface elements** to capture the interaction between the soil and adjacent structures.
- **Everything else** you are learning in this workshop (i.e., how to create beam elements, apply loads and boundary conditions, record results, perform the analysis, etc.



## Outline

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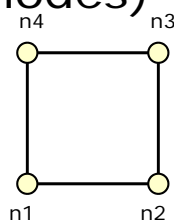
- Finite Elements (for solids)
  - Single-phase
  - Multi-phase (coupled) finite elements
  - Zero length element
- Material Models
  - Elastic
  - Elasto-plastic Continuum Models
  - Elasto-plastic Uniaxial models
- Boundary Conditions
  - Equal DOF
  - Absorbent boundaries

## Finite Elements (solids)

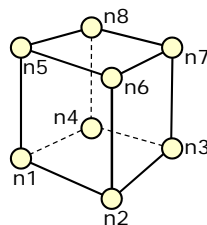
- Single-phase formulations
  - To capture the response of dry soils (or total stress analysis) → need one single phase
    - Phase 1 – soil skeleton
- Multi-phase formulations
  - To capture the response of saturated soils (effective stress analysis) → need two phases
    - Phase 1 → Soil skeleton
    - Phase 2 → pore water
- Zero-Length element
  - To capture interface response between solid and beam elements, and to apply absorbent boundary conditions

## Single Phase Formulations

- Small deformation solid elements
  - 2-D quadrilateral elements (4, 9 nodes)
  - 3-D solid elements, brick (8, 20 nodes)
- Large deformation (total Lagrangian) solid elements, bricks (20 nodes)

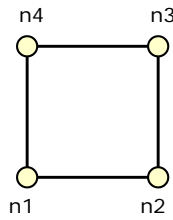


**quad** (4 node)



**stdBrick** (8 node)

## quad element definition



**quad** (4 node)

```
element quad $eleTag $n1 $n2 $n3 $n4 $thick $type $matTag
<$press $rho $b1 $b2>
```

Must define first all the required arguments. In particular:

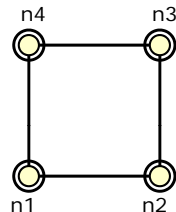
**Nodes** \$n1, \$n2, \$n3, \$n4 and

**Material type** \$matTag

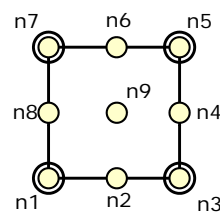
The arguments in <...> are optional

## Multi-Phase Formulations

- Fully coupled u-p elements (2D & 3D)
- Fully coupled u-p-U elements (3D) for small deformations



**quadUP**

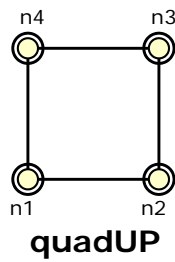


**9\_4\_quadUP**

Degrees of Freedom (DOFs) are:

- u → solid displacement, on ○
- P → pore fluid pressures, on ○
- U → pore fluid displacements, on ○

## quadUP element definition

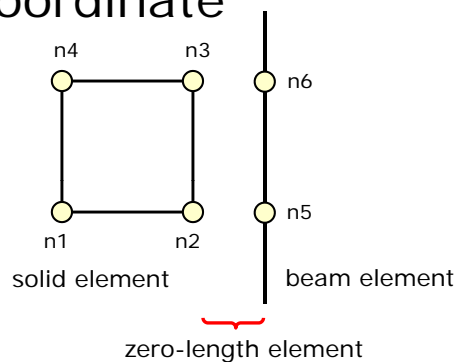


```
element quadUP $eleTag $n1 $n2 $n3 $n4 $thick $type $matTag  
$bulk $fmass $hPerm $vPerm <$b1 $b2 $t>
```

\$bulk → combined undrained bulk modulus  $B_c = B_f/n$   
\$fmass → fluid mass density  
\$hperm & \$vperm → horiz. And vert. permeability

## zerolength element

- Connects two points at the same coordinate



```
element zeroLength $eleTag $n1 $n2 -mat $matTag1 $matTag2 ...  
-dir $dir1 $dir2 ... <-orient $x1 $x2 $x3 $yp1 $yp2 $yp3>
```



## Material Models

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- Linear Elastic Material model (**nDMaterial**)
  - To characterize the response of the soil (or other continuum) in its elastic regime
- Elasto-Plastic Material models (**nDMaterial**)
  - To characterize the nonlinear stress-strain response of soils
- Elasto-plastic Uniaxial models
  - To characterize the interface response between soil and structural elements (**uniaxialMaterial**).



## nDMaterial Elastic

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- Small deformation elasticity
  - Linear isotropic
  - Nonlinear isotropic
  - Cross anisotropic
- Elastic Isotropic Material

**nDMaterial ElasticIsotropic** \$matTag \$E \$v

## nDMaterial Elasto-Plastic (Small Deformations)

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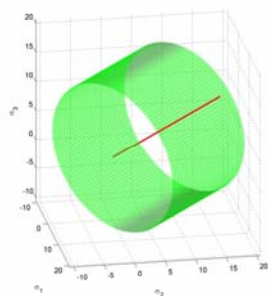
- J2-Plasticity Material (von Mises)
- Drucker-Prager Material (UW)
- Template Elasto-Plastic Material (UC Davis)
- Cam-Clay Material (Berkeley)
- MutiYield Materials (San Diego)
- FluidSolidPorous Material(SanDiego)

## nDMaterial J2Plasticity

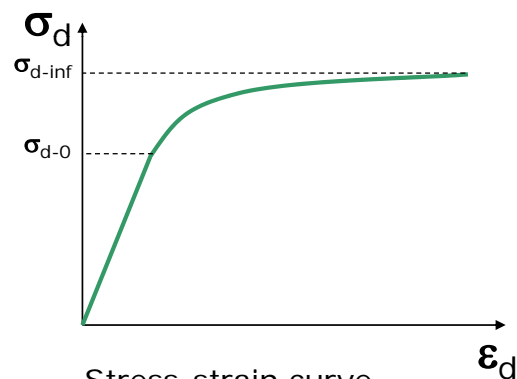
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- von-Mises type

nDMaterial J2Plasticity \$matTag \$K \$G \$sig0 \$sigInf \$delta \$H



Von-Mises Yield Surface



Stress-strain curve

## nDMaterial

### Template Elasto-Plastic Material

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- Versatile tool to generate multiple types of elasto-plastic materials by combining **yield surfaces**, **plastic potentials** and **evolution laws**
- Developed by Boris Jeremic at UC Davis  
<http://sokocalo.engr.ucdavis.edu/~jeremic>

## nDMaterial

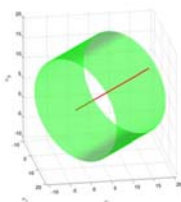
### Template Elasto-Plastic Material

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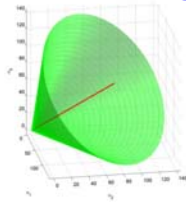
```
nDMaterial Template3Dep $matTag $ElmatTag  
-YS $ys -PS $ps -EPS $eps <-ELS1 $el> <-ELT1 $et>
```

- YS → **Yield surfaces** (von Mises, Drucker Prager, Mohr-Coulomb, Camclay)
- PS → **Plastic potentials** (von Mises, Drucker-Prager Mohr-Coulomb, Camclay, Leon)
- EPS → **Initial state of stress**
- ELS1 → **Scalar evolution laws** for isotropic hardening (linear, nonlinear Camclay)
- ELT1 → **Tensorial evolution laws** for Kinematic hardening (linear, nonlinear Armstrong-Frederick)

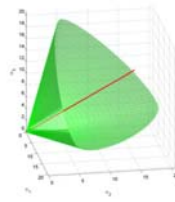
Von-Mises



Drucker Prager



Mohr-Coulomb



Camclay





## nDMaterial MultiYield Materials

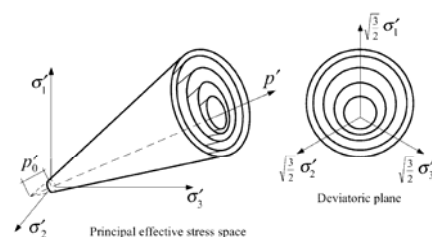
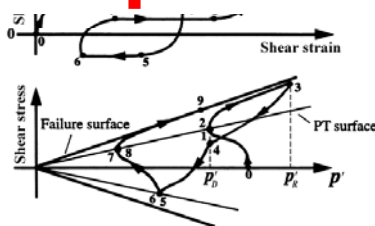
- Material models based on Multiyield Plasticity (*Mroz et al., Prevost et al.*)
- Two types
  - Pressure Independent Multi-yield (for total stress analysis)
  - Pressure Dependent Multi-yield (captures well the response of liquefiable soils)
  - Fluid-solid porous material (Material to couple solid & fluid phases)
- Developed by Elgamal et al. at UCSD  
<http://cyclic.ucsd.edu/opensees/>

## nDMaterial PressureDependentMultiYield

```

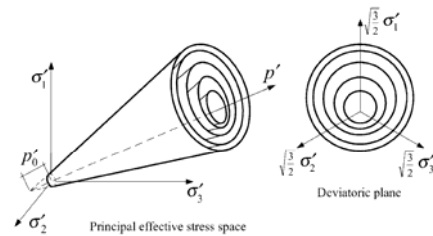
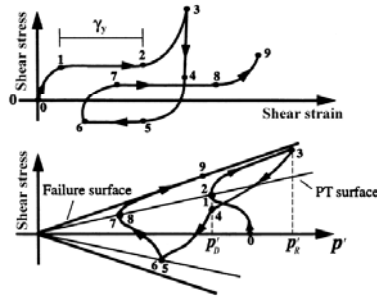
nDMaterial PressureDependMultiYield $matTag $nd $rho
$refShearModul $refBulkModul $frictionAng $peakShearStra
$refPress $pressDependCoe $PTAng
$contrac $dilat1 $dilat2, $liquefac1 $liquefac2 $liquefac3
<$noYieldSurf=20 <$r1 $Gs1 ...>
$e=0.6 $cs1=0.9 $cs2=0.02 $cs3=0.7 $pa=101>
  
```

# 15 parameters!!??



# nDMaterial PressureDependentMultiYield

**nDMaterial PressureDependMultiYield** \$matTag \$nd \$rho  
 \$refShearModul \$refBulkModul \$frictionAng \$peakShearStra  
 \$refPress \$pressDependCoe \$PTAng  
 \$contrac \$dilat1 \$dilat2, \$liquefac1 \$liquefac2 \$liquefac3  
 <\$noYieldSurf=20 <\$r1 \$Gs1 ...>  
 \$e=0.6 \$cs1=0.9 \$cs2=0.02 \$cs3=0.7 \$pa=101 >



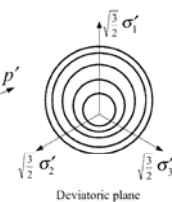
# nDMaterial PressureDependentMultiYield

nDMat  
 \$refShe  
 \$refPre  
 \$contra  
 <\$noYi  
 \$e=0.

	Loose Sand (15%-35%)	Medium Sand (35%-65%)	Medium-dense Sand (65%-85%)	Dense Sand (85%-100%)
$\rho$ (ton/m <sup>3</sup> )	1.7	1.9	2.0	2.1
$refShearModul$ (kPa, at $p'_r=80$ kPa)	$5.5 \times 10^4$	$7.5 \times 10^4$	$1.0 \times 10^5$	$1.3 \times 10^5$
$refBulkModu$ (kPa, at $p'_r=80$ kPa)	$1.5 \times 10^5$	$2.0 \times 10^5$	$3.0 \times 10^5$	$3.9 \times 10^5$
$frictionAng$	29	33	37	40
$peakShearStra$ (at $p'_r=80$ kPa)	0.1	0.1	0.1	0.1
$refPress$ ( $p'_r$ , kPa)	80	80	80	80
$pressDependCoe$	0.5	0.5	0.5	0.5
$PTAng$	29	27	27	27
$contrac$	0.21	0.07	0.05	0.03
$dilat1$	0.	0.4	0.6	0.8
$dilat2$	0	2	3	5
$liquefac1$ (kPa)	10	10	5	0
$liquefac2$	0.02	0.01	0.003	0
$liquefac3$	1	1	1	0
$e$	0.85	0.7	0.55	0.45

nd \$rho  
 earStra  
 ac3

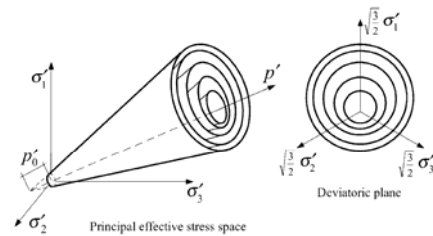
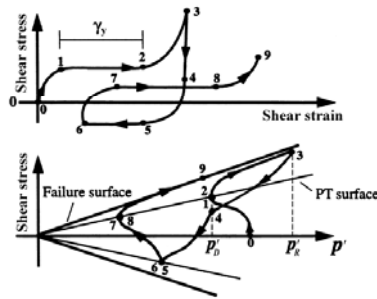
Shear stress  
 Shear stress



Principal effective stress space

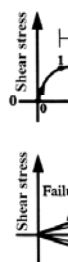
# nDMaterial PressureDependentMultiYield02

**nDMaterial PressureDependMultiYield02** \$matTag \$nd \$rho  
 \$refBulkModul \$frictionAng \$peakShearStra \$refPress  
 \$pressDepenCoe \$PTAng  
 \$contrac1 \$contrac3 \$dilat1 \$dilat3  
 <\$noYieldSurf=20 <\$r1 \$Gs1 ...>  
 \$contrac2=5.0 \$dilat2=3.0 \$liquefac1=1.0 \$liquefac2=0.0  
 \$e=0.6 \$cs1=0.9 \$cs2=0.02 \$cs3=0.7 \$pa=101>



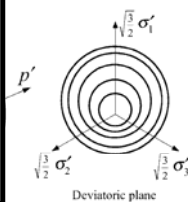
# nDMaterial PressureDependentMultiYield02

**nDMater** \$nd \$rho  
 \$refBulkM  
 \$pressDe  
 \$contrac1  
 <\$noYield  
 \$contrac2  
 \$e=0.6 \$



	Dr=30%	Dr=40%	Dr=50%	Dr=60%	Dr=75%
$\rho$ (ton/m <sup>3</sup> )	1.7	1.8	1.9	2.0	2.1
refShearModul (kPa, at $p'_i=80$ kPa)	$6 \times 10^4$	$9 \times 10^4$	$10 \times 10^4$	$11 \times 10^4$	$13 \times 10^4$
refBulkModu (kPa, at $p'_i=80$ kPa)	$16 \times 10^4$ ( $K_0=0.5$ )	$22 \times 10^4$ ( $K_0=0.47$ )	$23.3 \times 10^4$ ( $K_0=0.45$ )	$24 \times 10^4$ ( $K_0=0.43$ )	$26 \times 10^4$ ( $K_0=0.4$ )
frictionAng	31	32	33.5	35	36.5
PTAng	31	26	25.5	26	26
peakShearStra (at $p'_i=101$ kPa)	0.1				
refPress ( $p'$ , kPa)	101				
pressDepenCoe	0.5				
Contrac1	0.087	0.067	0.045	0.028	0.013
Contrac3	0.18	0.23	0.15	0.05	0.0
dilat1	0.	0.06	0.06	0.1	0.3
dilat3	0.0	0.27	0.15	0.05	0.0
e	0.85	0.77	0.7	0.65	0.55

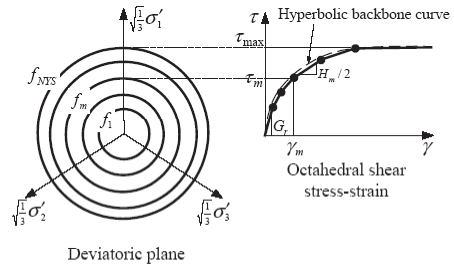
\$nd \$rho  
 =0.0



Principal effective stress space

# nDMaterial PressureIndependentMultiYield

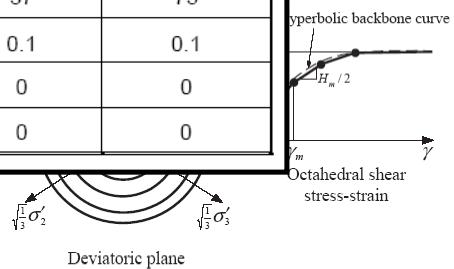
**nDMaterial PressureIndependentMultiYield** \$matTag \$nd \$rho  
 \$refShearModul \$refBulkModul \$cohesi \$peakShearStra  
 \$frictionAng \$refPress=101 \$pressDependCoe=0.  
 <\$noYieldSurf=20 <\$r1 \$Gs1 ...>>



# nDMaterial PressureIndependentMultiYield

**nDMaterial PressureIndependentMultiYield** \$matTag \$nd \$rho  
 \$refShearModul \$refBulkModul \$cohesi \$peakShearStra  
 \$frictionAng \$refPress=101 \$pressDependCoe=0.  
 <\$noYieldSurf=20 <\$r1 \$Gs1 ...>>

	Soft Clay	Medium Clay	Stiff Clay
<i>rho</i> (ton/m <sup>3</sup> )	1.3	1.5	1.8
<i>refShearModul</i> (kPa)	1.3x10 <sup>4</sup>	6.0x10 <sup>4</sup>	1.5x10 <sup>5</sup>
<i>refBulkModu</i> (kPa)	6.5x10 <sup>4</sup>	3.0x10 <sup>5</sup>	7.5x10 <sup>5</sup>
<i>cohesi</i> (kPa)	18	37	75
<i>peakShearStra</i>	0.1	0.1	0.1
<i>frictionAng</i>	0	0	0
<i>pressDependCoe</i>	0	0	0





## nDMaterial FluidSolidPorousMaterial

---

- Couples the response of two phases: fluid and solid – developed to simulate the response of saturated porous media

```
nDMaterial FluidSolidPorousMaterial $matTag $nd  
$soilMatTag $combinedBulkModul
```

\$soilMatTag → the tag of previously defined material  
\$combinedBulkModul → combined undrained bulk modulus,  
 $B_c = B_f/n$



## Additional commands for **multiyield** materials

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- Help perform stage analysis

```
updateMaterialStage –material $matTag –stage $sNum
```

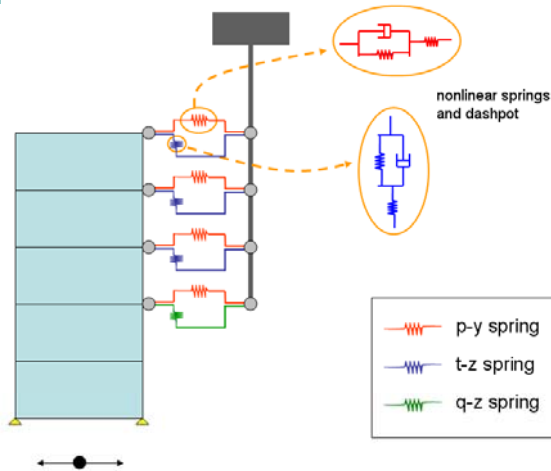
\$MatTag → the tag of previously defined material  
\$sNum → (0 - elastic, 1-plastic, 2 – linear elastic constant  $f(\sigma_3)$  )

```
updateParameter –material $matTag –refG $newVal
```

\$MatTag → the tag of previously defined material  
\$sNewVal → new parameter value

# Elasto-plastic Uniaxial models

- To capture interface response between solid (soil) and beam elements (pile)



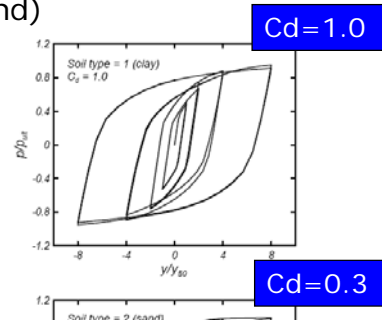
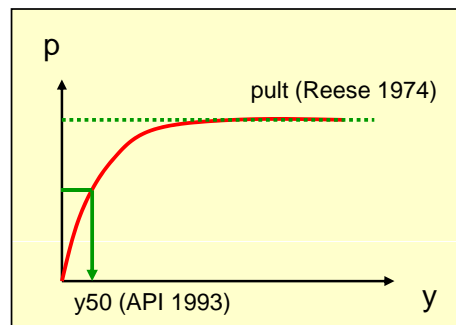
Py Tz Qz Uniaxial Materials

- PySimple1
- TzSimple1
- QzSimple1
- PyLiq1
- TzLiq1

## uniaxialMaterial PySimple1

```
uniaxialMaterial PySimple1 matTag $soilType $pult $Y50 $Cd
<$c>
```

- \$soilType → =1 Matlock (clay), =2 API (sand)
- \$pult → ultimate capacity of p-y material
- \$Y50 → displ. @ 50% of pult
- Cd → drag resistance (=1 no gap, <1 gap)
- \$c → viscous damping





## uniaxialMaterial TzSimple1 & QzSimple1

```
uniaxialMaterial TzSimple1 matTag $tzType $tult $z50 <$c>
```

\$tzType → =1 Reese & O'Neill (clay), =2 Mosher (sand)  
\$tult → ultimate capacity of t-z material  
\$z50 → displ. @ 50% of tult  
\$c → viscous damping

```
uniaxialMaterial QzSimple1 matTag $qzType $qult $z50  
<$suction $c>
```

\$qzType → =1 Reese & O'Neill (clay), =2 Vijayvergiya (sand)  
\$qult = ultimate capacity of q-z material  
\$z50 = displ. @ 50% of qult  
\$suction → uplift resistance = suction\*qult  
\$c viscous damping

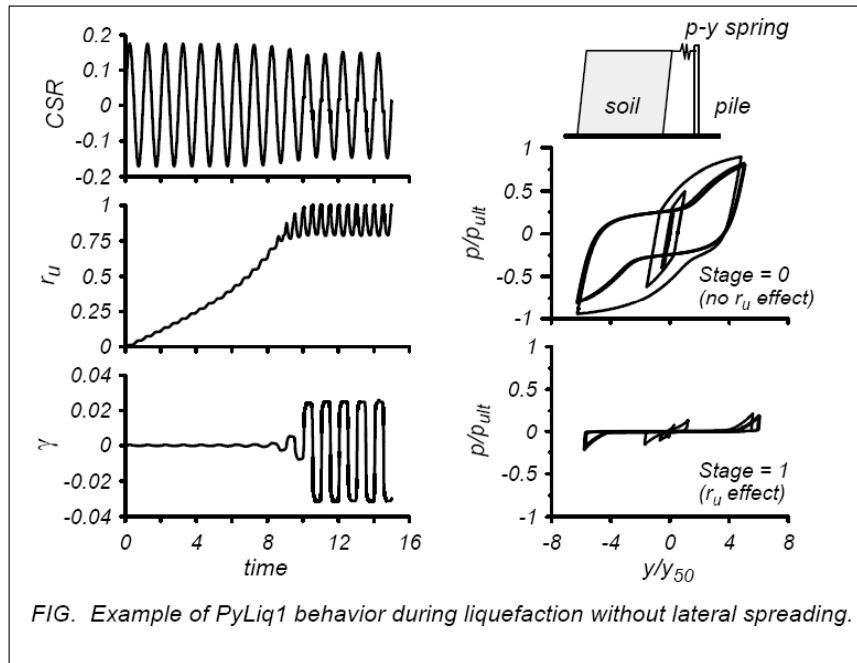


## uniaxialMaterial PyLiq1

```
uniaxialMaterial PyLiq1 $matTag $soilType $pult $Y50 $Cd $c  
$pRes $solidElem1 $solidElem2
```

\$soilType → =1 Matlock (clay), =2 API (sand)  
\$pult → ultimate capacity of p-y material  
\$Y50 → displ. @ 50% of pult  
Cd → drag resistance (=1 no gap, <1 gap)  
\$c → viscous damping  
\$pRes → residual (minimum) p-y resistance as  $r_u=1.0$   
\$solidElem1 & \$solidElem2 → solid elements from which PyLiq1  
will obtain effective stresses and pore pressures

# uniaxialMaterial PyLiq1



## Boundary Conditions

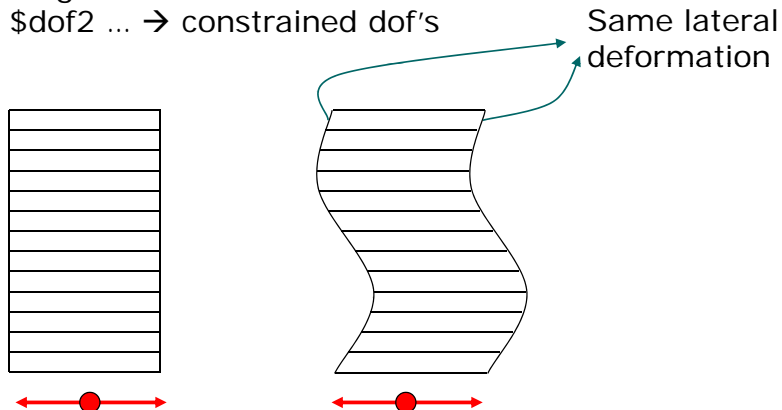
### ○ EqualDof

```
equalDOF $rNodeTag $cNodeTag $dof1 $dof2 ...
```

\$rNodeTag → master node

\$cNodeTag → slave node

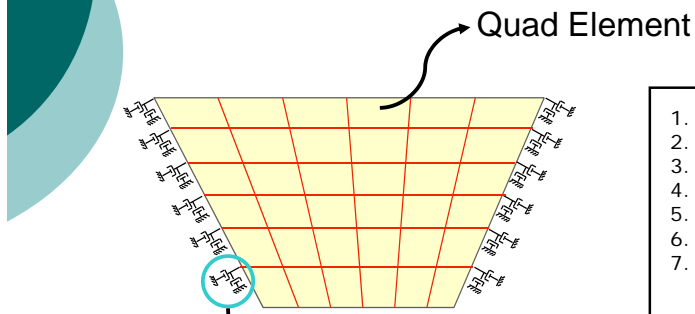
\$dof1 \$dof2 ... → constrained dof's



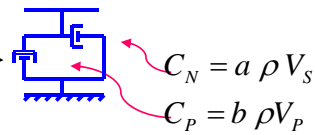


# Absorbent Boundaries Lysmer (1969)

---



```
1. set DampP 755
2. set DampN 1216
3. uniaxialMaterial Elastic 1 0 $DampP
4. uniaxialMaterial Elastic 2 0 $DampN
5. node 1 16.0 0.0
6. node 2 16.0 0.0
7. element zeroLength 1 1 2 -mat 1 2
   -dir 1 2 -orient 1 -2 0 2 1 0
```



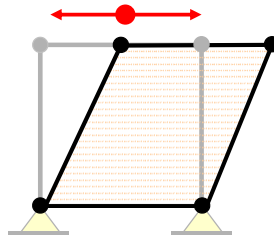
zeroLength Element &  
uniaxial material

---

Many more capabilities currently  
under development!!

# Basic Example

- Response of saturated soil element to harmonic excitation



```
#Created by Zhaohui Yang (zhyang@ucsd.edu)
#plastic pressure dependent material
#plane strain, single element, dynamic analysis (input motion: sinusoidal acceleration at base)
#SI units (m, s, KN, ton)
```

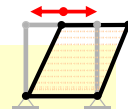
```
#
#      4   3
#      |   |
#      |   |
#      |   |
#      1-----2 (nodes 1 and 2 fixed)
#      ^     ^
#      <--> input motion: sinusoidal acceleration at base
wipe
#
```

**Define basic variables  
using good names!!!**

```
#some user defined variables
```

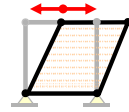
```
#
set accMul 2 ;# acceleration multiplier
set massDen 2.0 ;# solid mass density
set fluidDen 1.0 ;# fluid mass density
set massProportionalDamping 0.0 ;
set stiffnessProportionalDamping 0.001 ;
set fangle 31.40 ;#friction angle
set prangle 26.50 ;#phase transformation angle
set E 90000.0 ;#shear modulus
set poisson 0.40 ;
set G [expr $E/(2*(1+$poisson))];
set B [expr $E/(3*(1-2*$poisson))];
set press 0.0 ;# isotropic consolidation pressure on quad element(s)
set deltaT 0.010 ;# time step for analysis
set numSteps 2000 ;# Number of analysis steps
set gamma 0.600 ;# Newmark integration parameter
set period 1 ;# Period of applied sinusoidal load
set pi 3.1415926535 ;
```

```
set inclination 0 ;
set unitWeightX [expr ($massDen-$fluidDen)*9.81*sin($inclination/180.0*$pi)] ;# unit weight in X direction
set unitWeightY [expr -($massDen-$fluidDen)*9.81*cos($inclination/180.0*$pi)] ;# unit weight in Y direction
```





# Define model geometry, materials & fixities



```
#####
#####
# create the ModelBuilder
model basic -ndm 2 -ndf 2

# define material and properties
nDMaterial PressureDependMultiYield 2 2 $massDen $G $B $fangle .1 80 0.5 \
    $ptangle 0.17 0.4 10 10 0.015 1.0
nDMaterial FluidSolidPorous 1 2 2 2.2D+6

updateMaterialStage -material 1 -stage 0
updateMaterialStage -material 2 -stage 0

# define the nodes
node 1 0.0D0 0.0D0
node 2 1.0D0 0.0D0
node 3 1.0D0 1.0D0
node 4 0.0D0 1.0D0

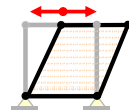
# define the element thick material maTag press mDensity gravity
element quad 1 1 2 3 4 1.0 "PlaneStrain" 1 $press 0. $unitWeightX $unitWeightY

# fix the base
fix 1 1 1
fix 2 1 1

# tie nodes 3 and 4
equalDOF 3 4 1 2
```



# Define gravity step VERY IMPORTANT!!!



```
#####
# GRAVITY APPLICATION (elastic behavior)

# create the SOE, ConstraintHandler, Integrator, Algorithm and Numberer
system ProfileSPD
test NormDispIncr 1.D-12 25 0
constraints Transformation
integrator LoadControl 1 1 1 1
algorithm Newton
numberer RCM

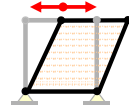
# create the Analysis
analysis Static

# analyze
analyze 2

# switch the material to plastic
updateMaterialStage -material 1 -stage 1
updateMaterialStage -material 2 -stage 1
updateParameter -material 2 -refB [expr $G*2/3.];

# analyze
analyze 1
```

# Define dynamic step



```
#####
# NOW APPLY LOADING SEQUENCE AND ANALYZE (plastic)
#####
```

```
# rezero time
setTime 0.0
wipeAnalysis
```

```
#create loading pattern
pattern UniformExcitation 1 1 -accel "Sine 0 1000 $period -factor $accMul"
```

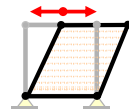
```
# create the Analysis
constraints Transformation;
test NormDisplncr 1.D-12 25 0
algorithm Newton
numberer RCM
system ProfileSPD
integrator Newmark $gamma [expr pow($gamma+0.5, 2)/4] \
    $massProportionalDamping 0.0 $stiffnessProportionalDamping 0.0
analysis VariableTransient
```

```
#create Recorders
recorder Node -file disp.out -time -node 1 2 3 4 -dof 1 2 -dT 0.01 disp
recorder Node -file acce.out -time -node 1 2 3 4 -dof 1 2 -dT 0.01 accel
recorder Element -ele 1 -time -file stress1.out material 1 stress -dT 0.01
recorder Element -ele 1 -time -file strain1.out material 1 strain -dT 0.01
recorder Element -ele 1 -time -file stress3.out material 3 stress -dT 0.01
recorder Element -ele 1 -time -file strain3.out material 3 strain -dT 0.01
recorder Element -ele 1 -time -file press1.out material 1 pressure -dT 0.01
recorder Element -ele 1 -time -file press3.out material 3 pressure -dT 0.01
```

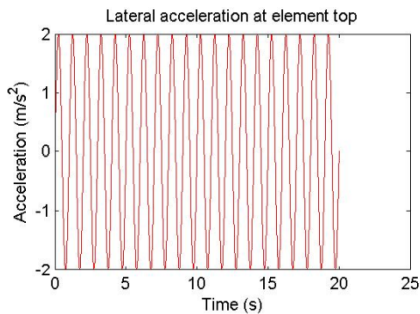
```
#analyze
set startT [clock seconds]
analyze $numSteps $deltaT [expr $deltaT/100] $deltaT 10
set endT [clock seconds]
puts "Execution time: [expr $endT-$startT] seconds."

wipe #flush ouput stream
```

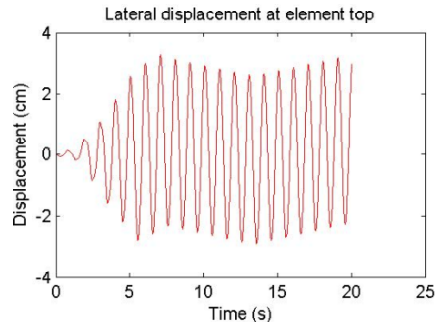
# Plot Results



Input Accel time history

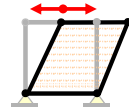


Output displ. time history

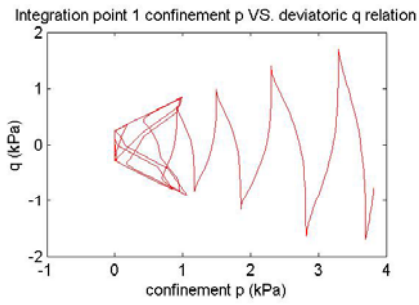
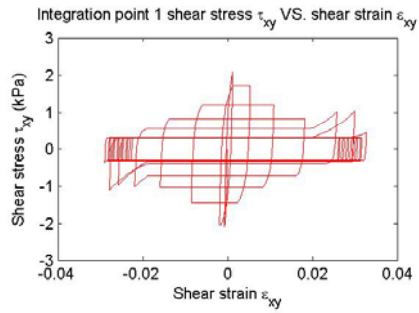




# Plot results



## Stress-strain & stress path



## Pore pressures

